

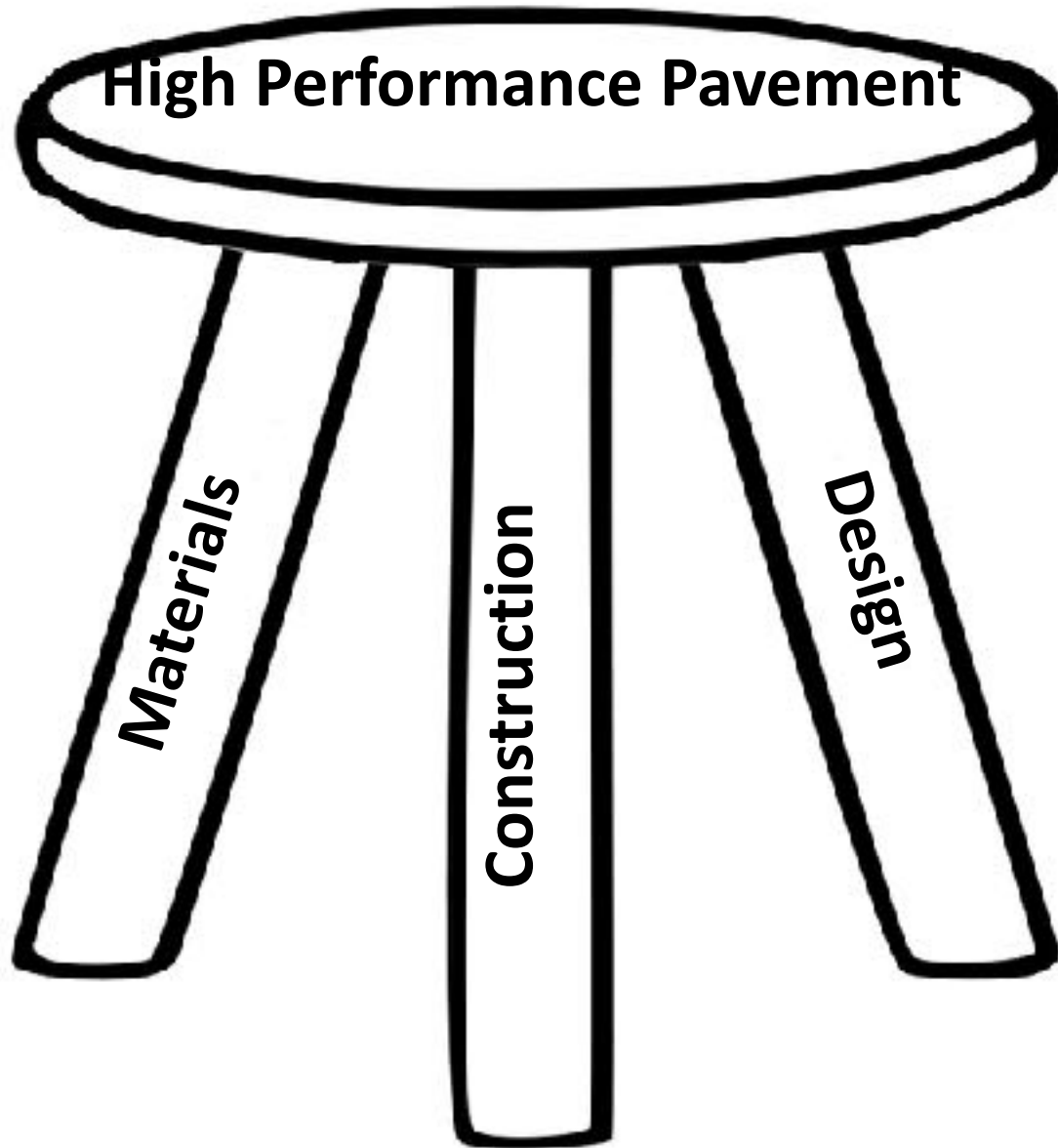


Presented By:
Mike Maloney, PE

October 18, 2017

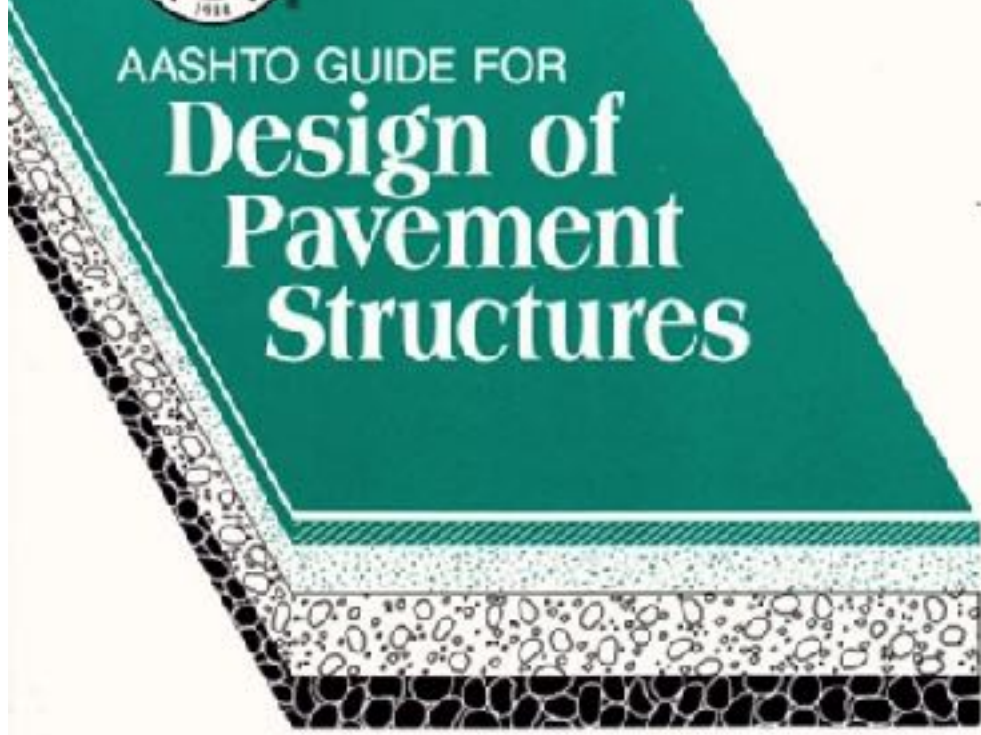
FALLING WEIGHT DEFLECTOMETER

Practical ♦ Cost effective ♦ Innovative uses





AASHTO GUIDE FOR
**Design of
Pavement
Structures**



PUBLISHED BY THE
AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS

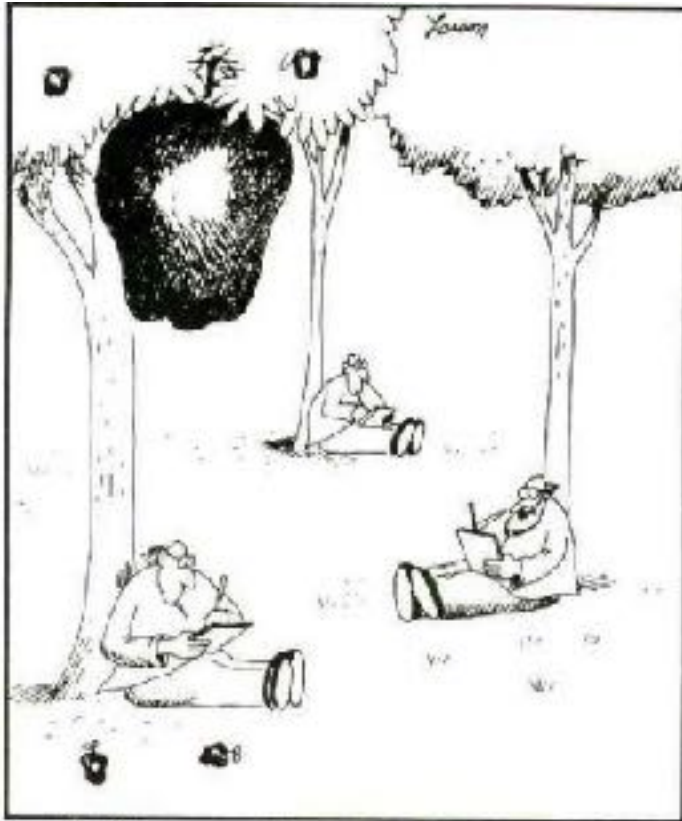




What is a Falling Weight Deflectometer (FWD)?

- Non-destructive pavement loading device
- Imparts a load impulse to the pavement structure
- Simulates a moving wheel load
- Measures deflections at the pavement surface





"Nothing yet. ... How about you, Newton?"

$$F = \sqrt{2Mghk}$$

Where:

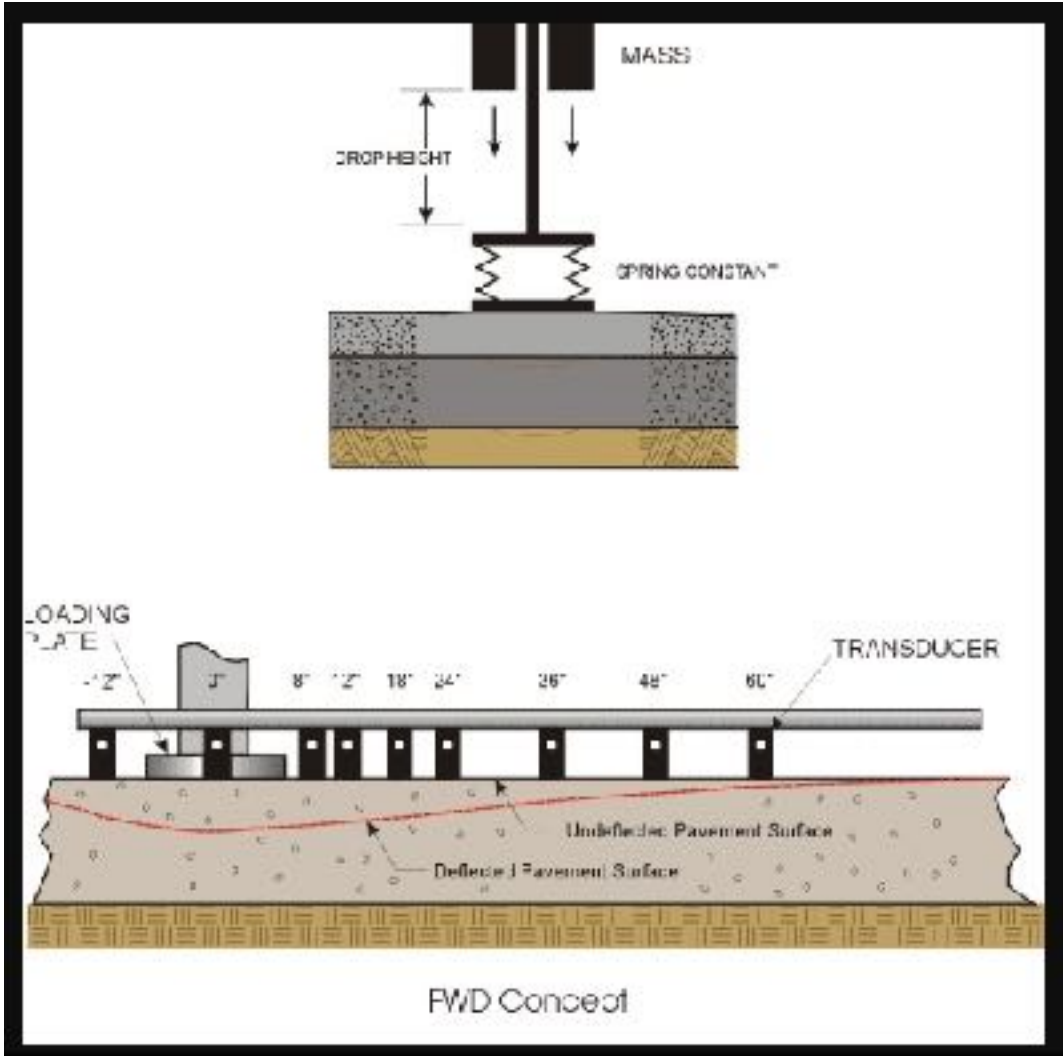
- M = mass of the falling
weight;
g = acceleration of gravity;
h = drop height;
K = spring constant.

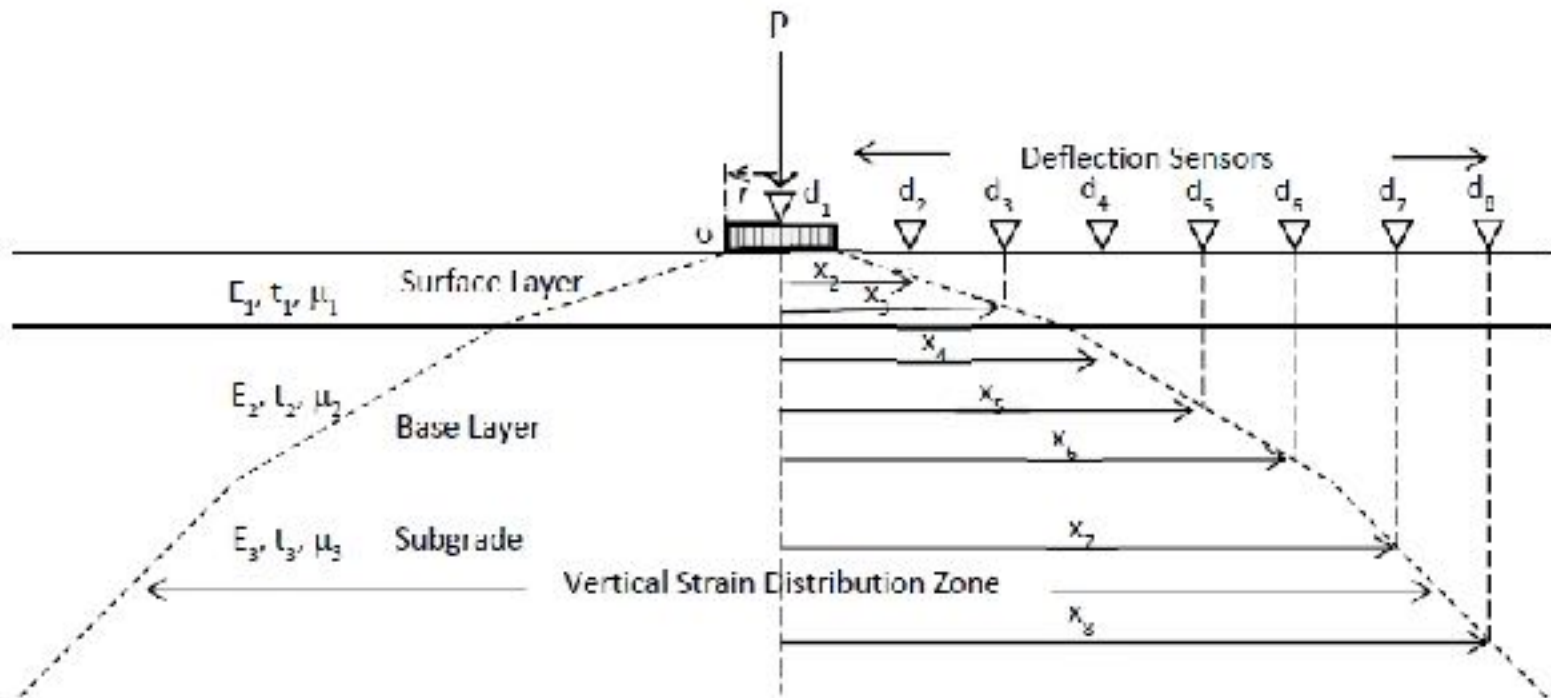
There are 3 possible ways to change the force:

- change the mass of the falling weight
- change the drop height
- change the spring constant

Only alternative "b" is feasible for the FWD (although alternative "a" is used at times).

Note that force is measured by the load cell and not calculated using the above equation.





What is the FWD used for?

- Estimate the structural capacity of the pavement
 - Overlay and Inlay Analysis
 - Analysis of remaining life and allowable traffic loading
 - Estimate of the subgrade & pavement layer elastic moduli values
 - Provides the ability to evaluate rehabilitation alternatives
- Evaluate the uniformity of support and identification of weak areas
- Detection of voids beneath rigid pavements
- Measurement of joint efficiency in rigid pavements

FWD Components

- Load Cell
- LVDT or Geophones
 - Deflection measurement
- Infrared temperature gages
 - Air temperature
 - Pavement surface temperature
- Electronic distance measurement
- Power source
- Control/data acquisition unit

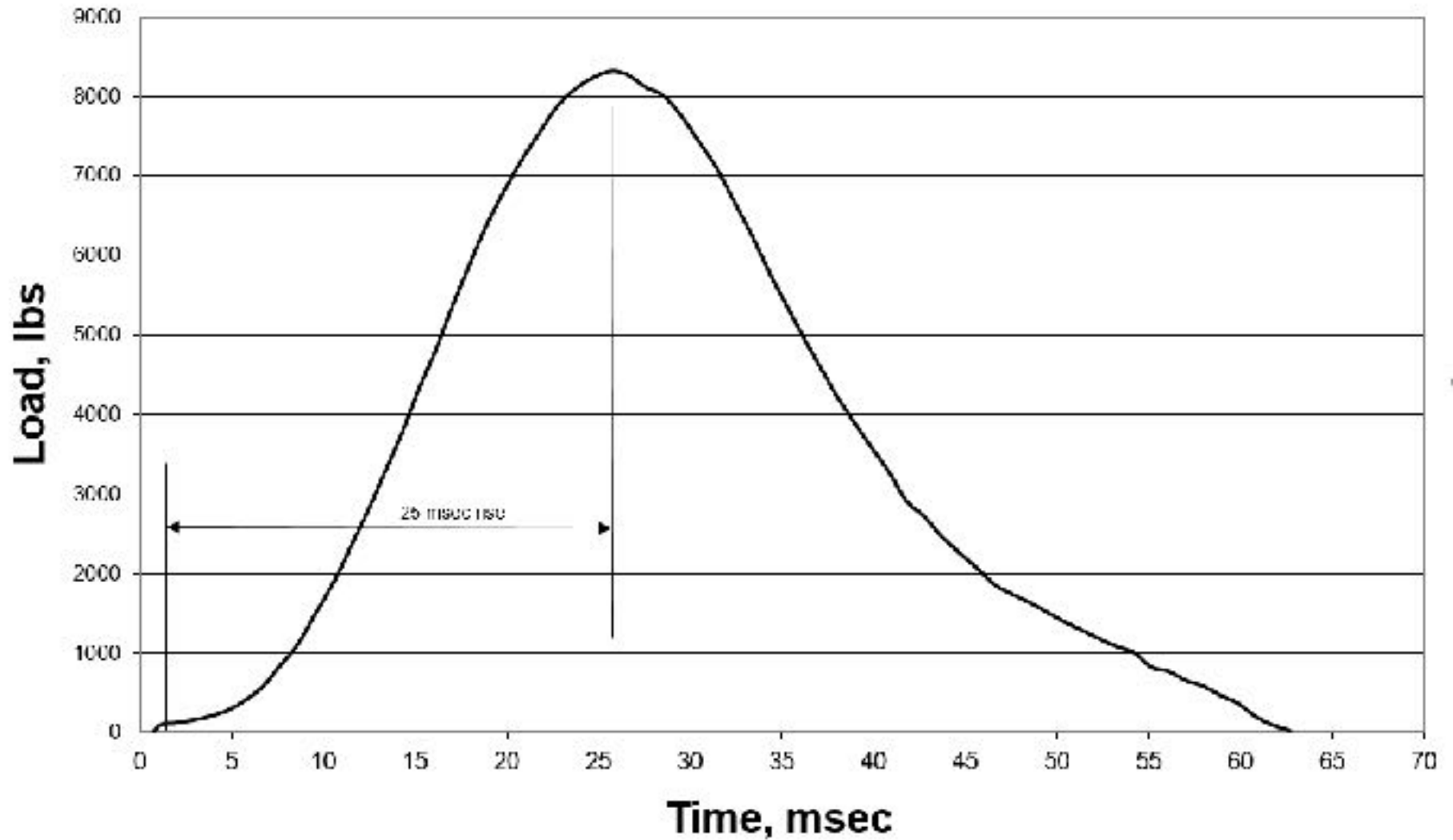
Advantages of FWD Testing

- Non-destructive test
- Wide coverage since testing can occur quickly
 - Typically 1 to 2 minutes per test
 - Can easily test different pavement conditions (for example difference between the inner and outer wheel path)
- Data compiled and viewed in real-time
- Gives an accurate assessment of the in situ properties of the pavement structure and subgrade

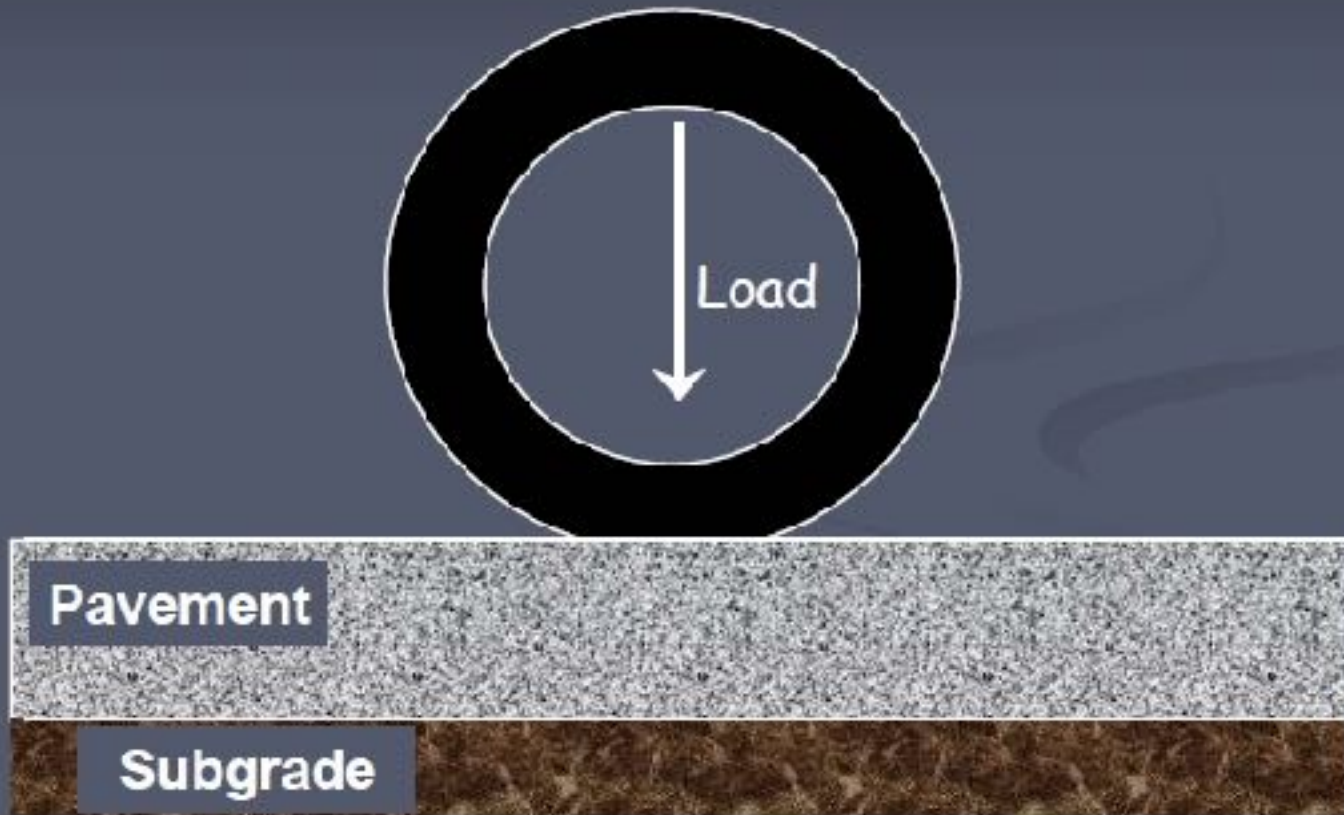
Limitations of FWD Testing

- The impact load must be adjusted to match the dynamic load of a heavy vehicle
- Analysis and interpretation of the results requires engineering judgement

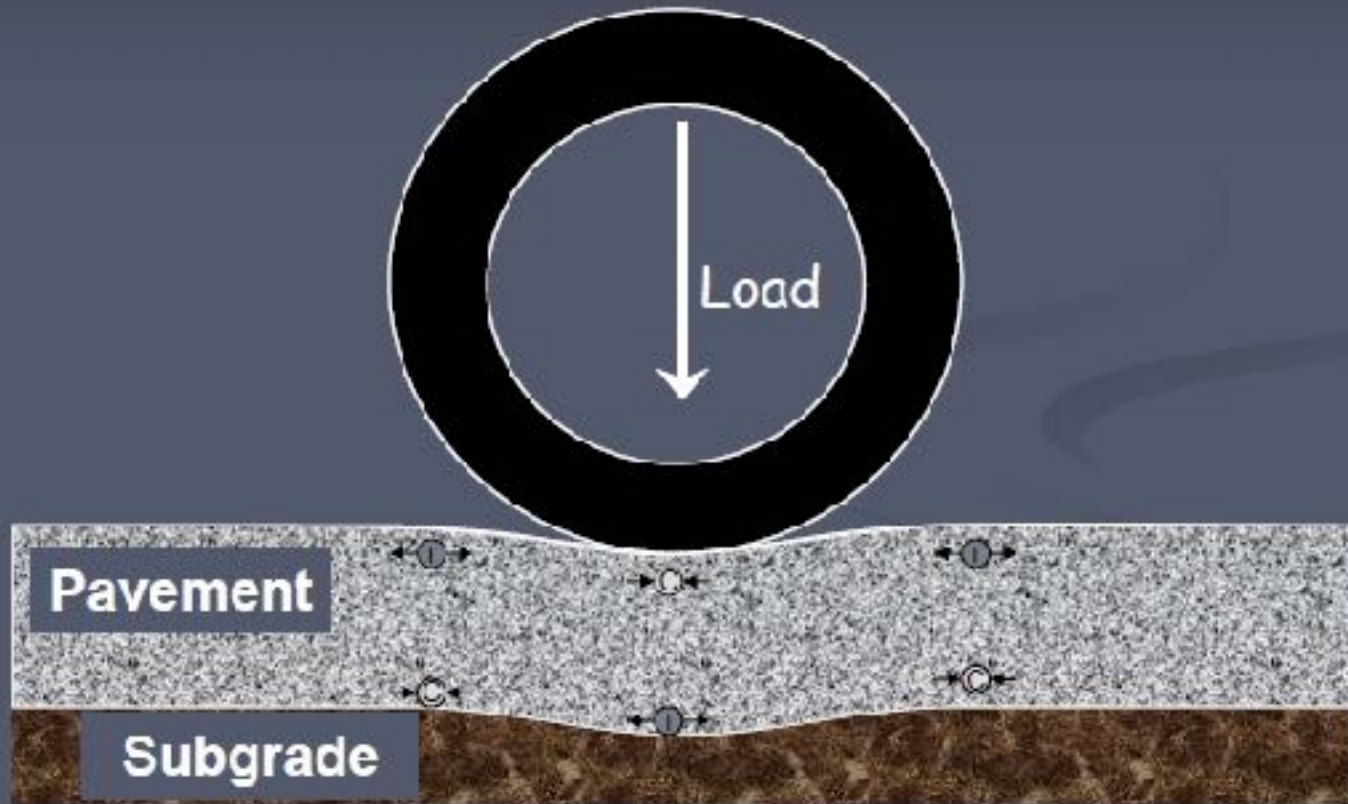
FWD Load Pulse



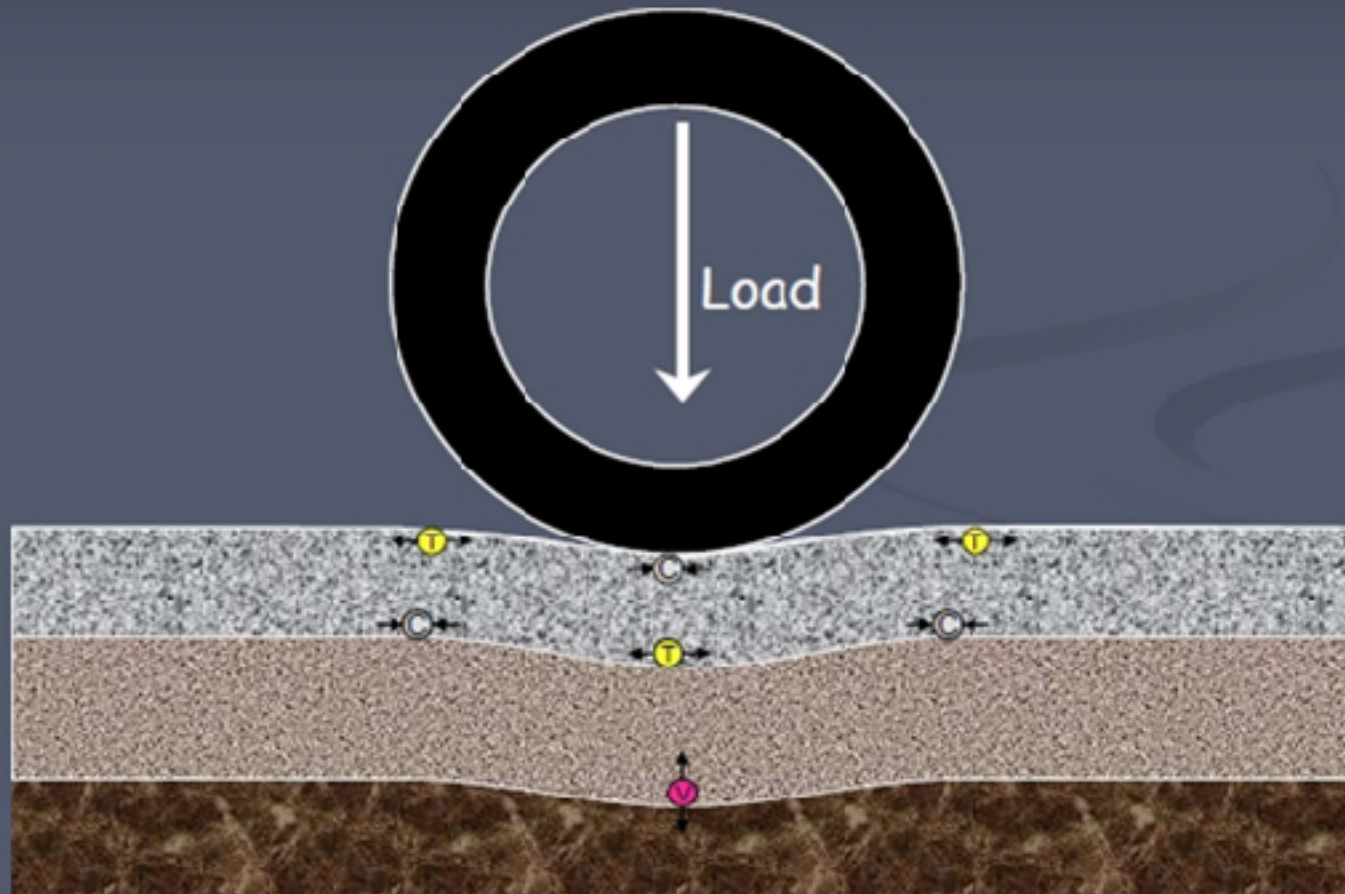
Pavement Deflection



Pavement Deflection

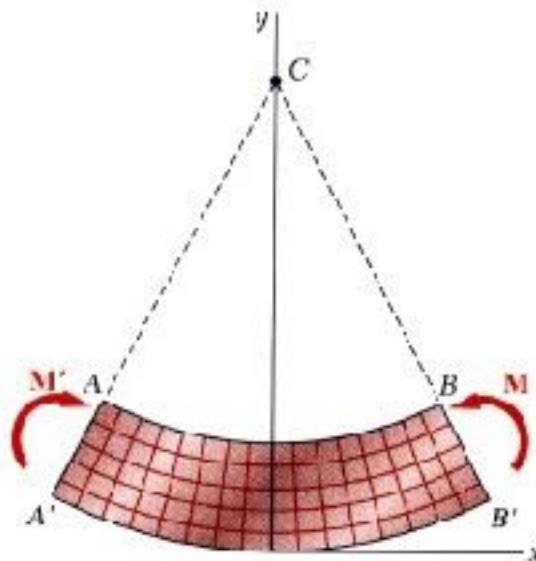


Locations of Critical Strains

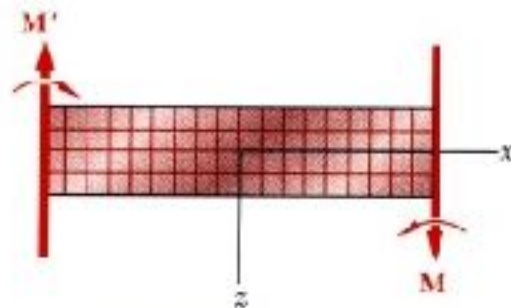


MECHANICS OF MATERIALS

Bending Deformations



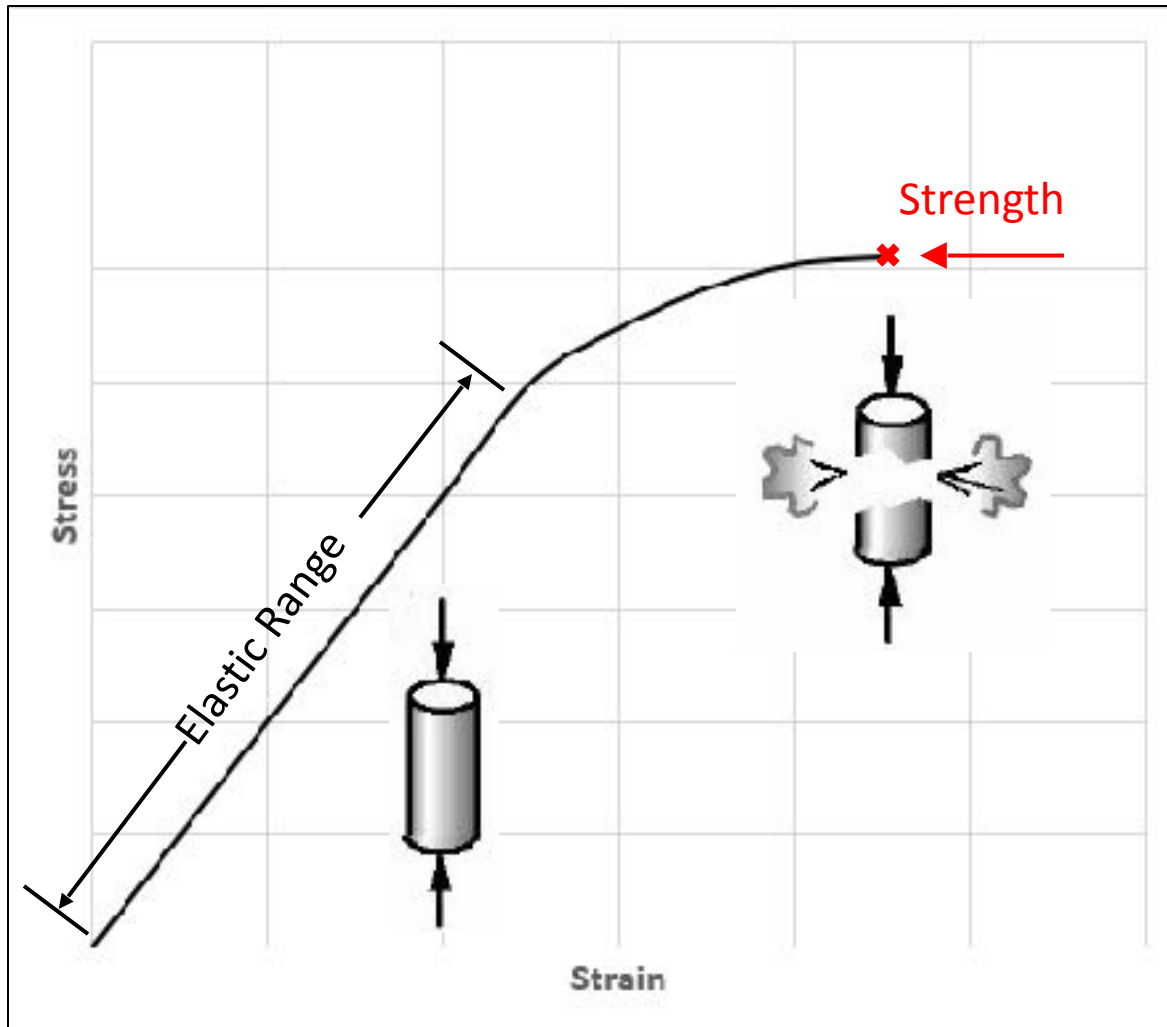
(a) Longitudinal, vertical section
(plane of symmetry)



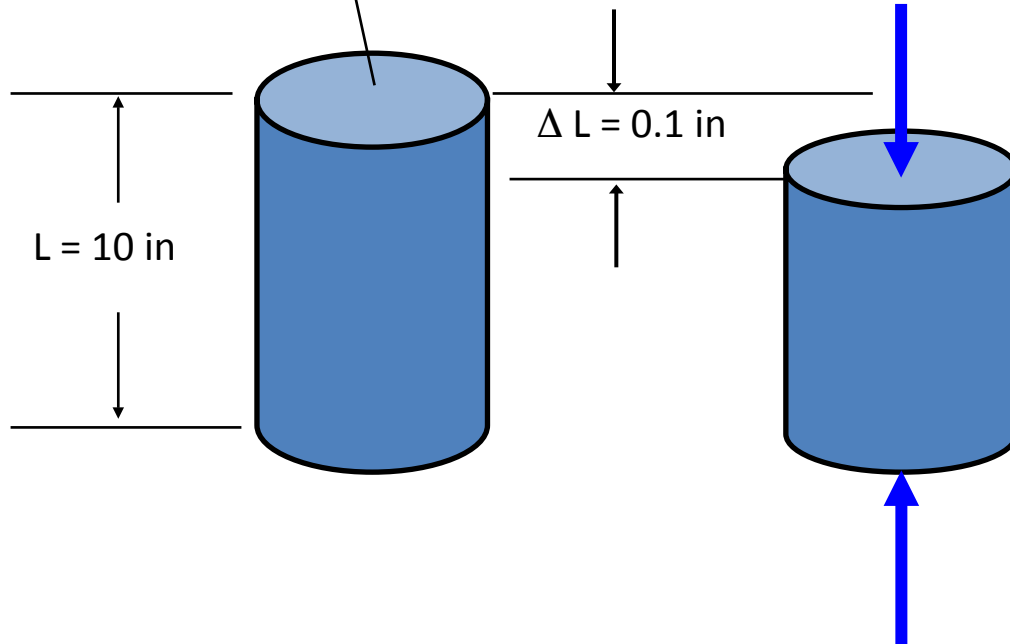
(b) Longitudinal, horizontal section

Beam with a plane of symmetry in pure bending:

- member remains symmetric
- bends uniformly to form a circular arc
- cross-sectional plane passes through arc center and remains planar
- length of top decreases and length of bottom increases
- a *neutral surface* must exist that is parallel to the upper and lower surfaces and for which the length does not change
- stresses and strains are negative (compressive) above the neutral plane and positive (tension) below it



Area = 10 in²



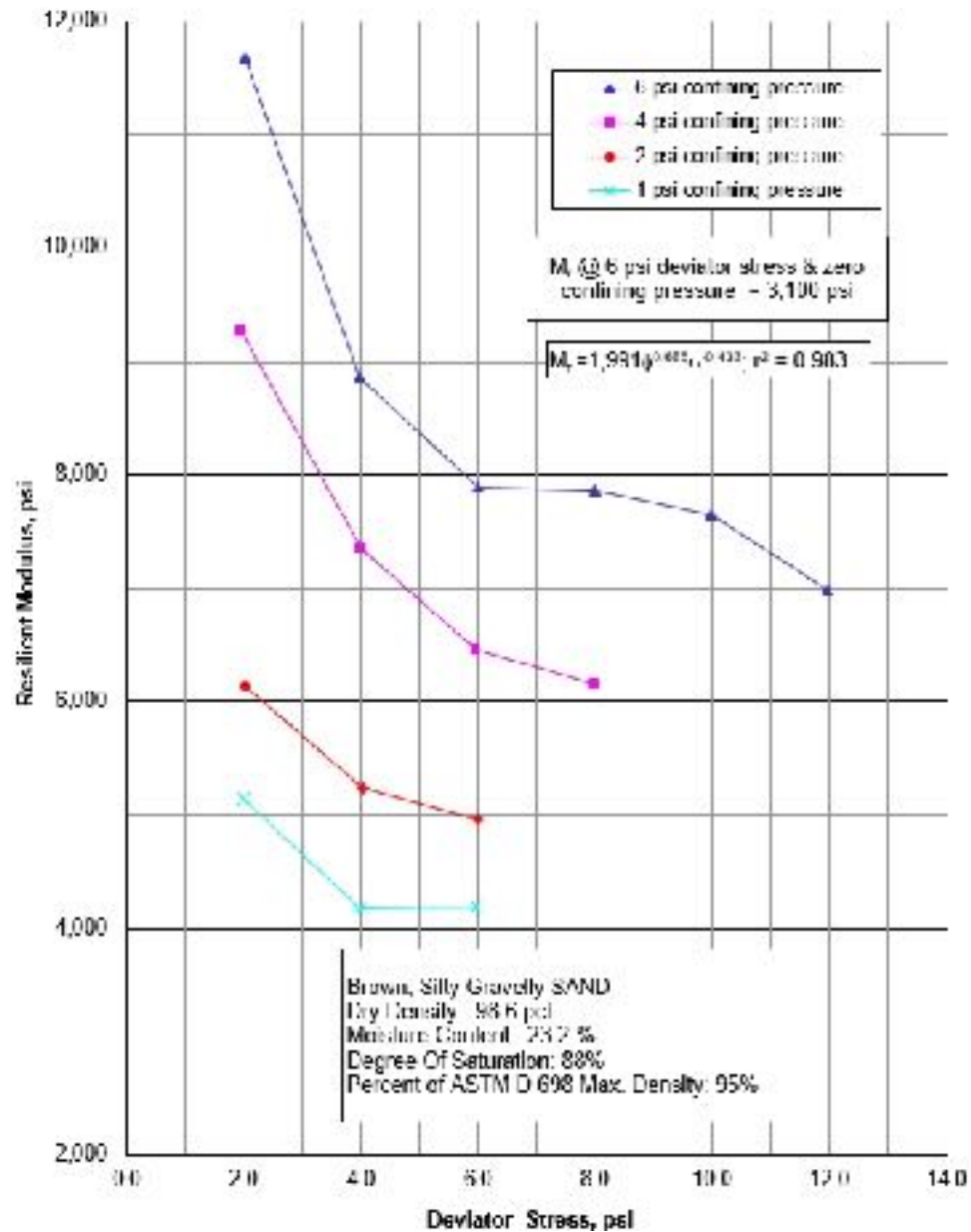
$$\text{Stress} = 1000/10 = 100 \text{ psi}$$

$$\text{Strain} = 0.1/10 = 0.01 \text{ in/in}$$

$$M_r = \text{Stress} / \text{Strain, psi}$$

$$M_r = 100 / 0.01 = 10,000 \text{ psi}$$

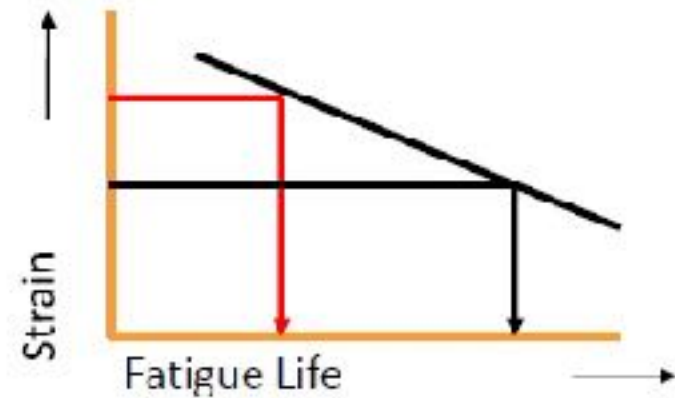
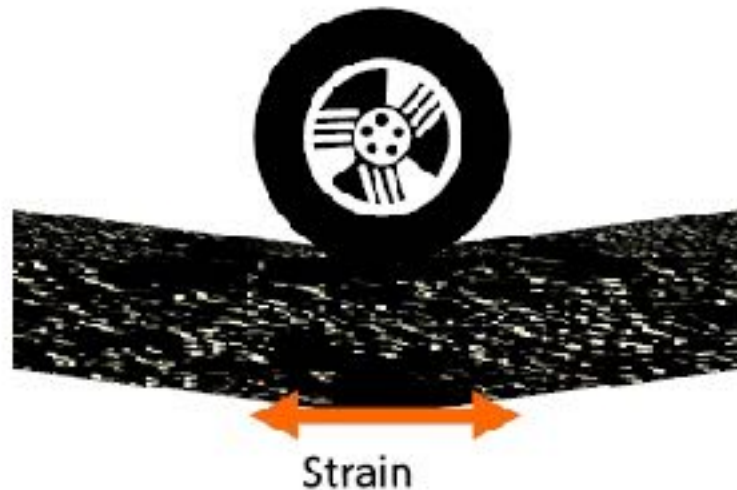
Triaxial Resilient Modulus Test Results - Figure 6
 (Bulk sample from Boring B-3 compacted to 95% of standard Proctor)



Fatigue Theory

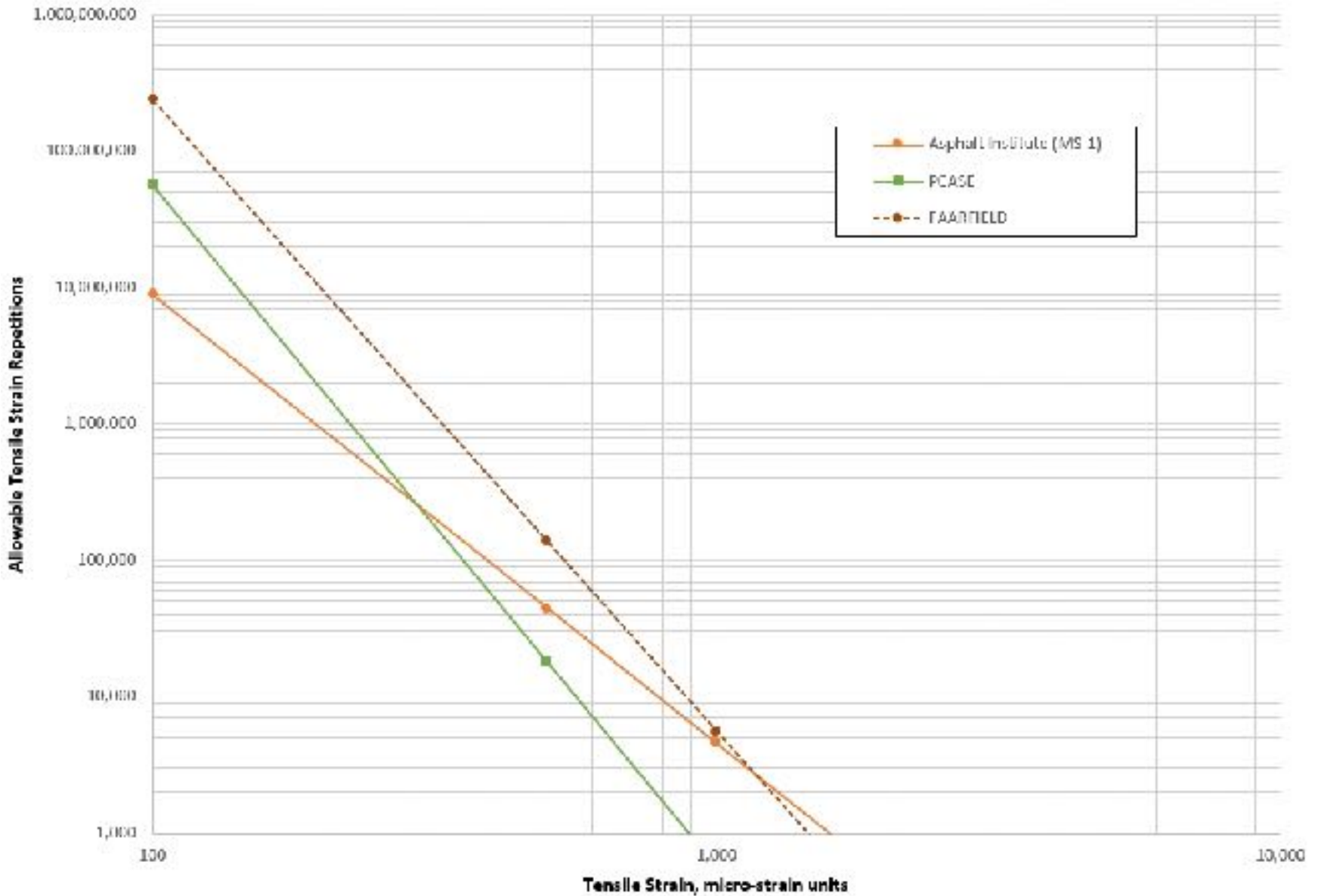
High Strain = Short Life

Low Strain = Long Life

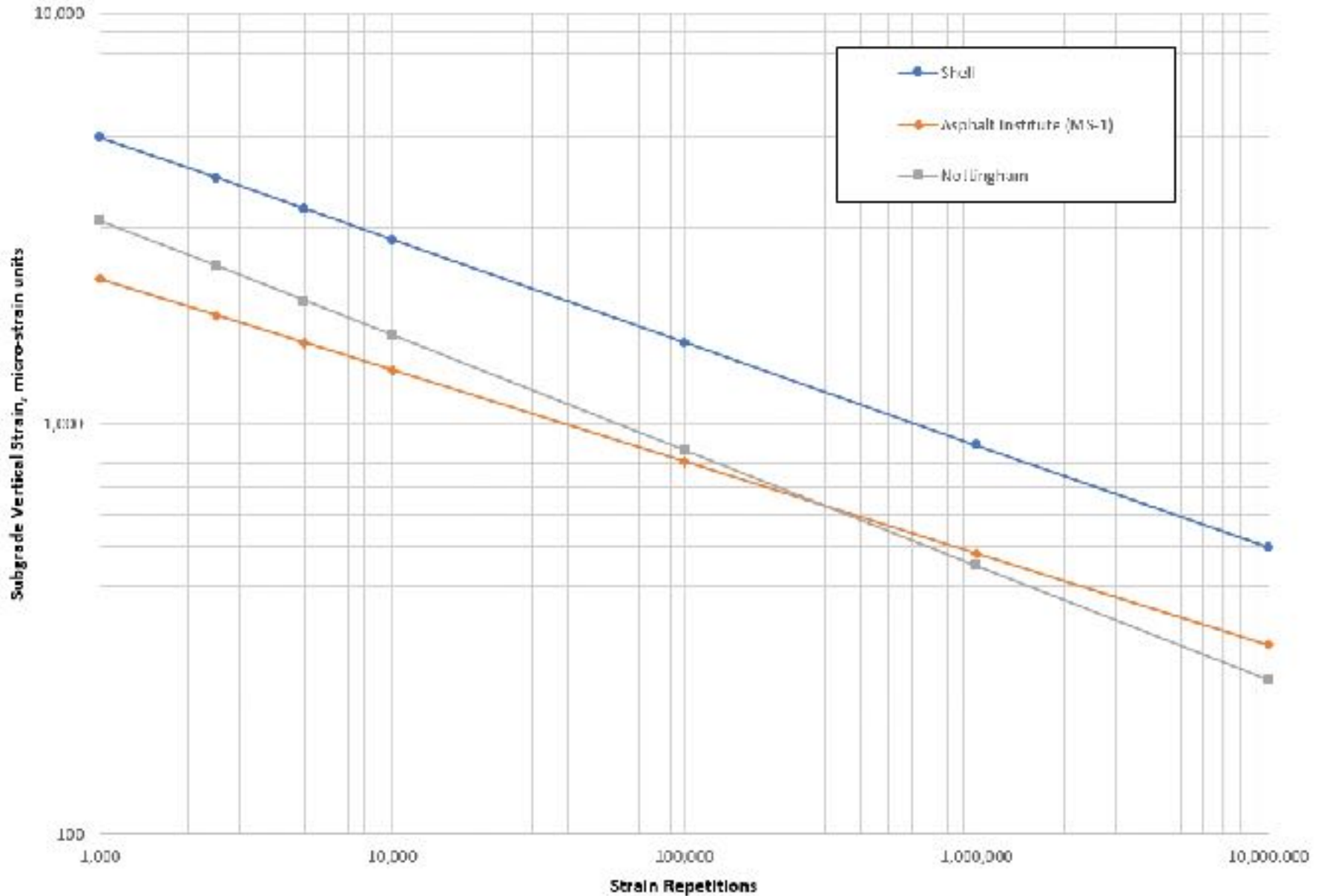


Extrapolations of loads from AASHO Road Test

Allowable Tensile Strain Repetitions Comparison



Allowable Subgrade Strain Comparison



Comparison of Allowable Asphalt Concrete Tensile Strain Repetitions Criteria for Given Strain Values

Asphalt Institute (MS-1) Tensile Strain Fatigue Criteria

Allowable tensile strain repetitions, N_f , based on NCHRP 201 fatigue relationship with failure defined as alligator cracking in 45% of wheelpath and with adjustment for effect of air voids per Asphalt Institute's Research Report No. 82-2, 8/82, as shown by the following equation:

$$N_f = F_1 * C_m * F_2 * s_t^{-F_3} * E_{ac}^{-F_4}$$

s_t = tensile strain at bottom of AC layer, in/in (or mm/mm)

E_{ac} = dynamic elastic modulus of AC, psi

F_1 = 18.4 Shift factor for difference between laboratory repetitions to failure and field repetitions to failure (corresponding to 45% cracking in wheel paths of AASHO Test Track)

F_2 = 0.00432 regression constant

F_3 = -1.291 regression constant

F_4 = -0.854 regression constant

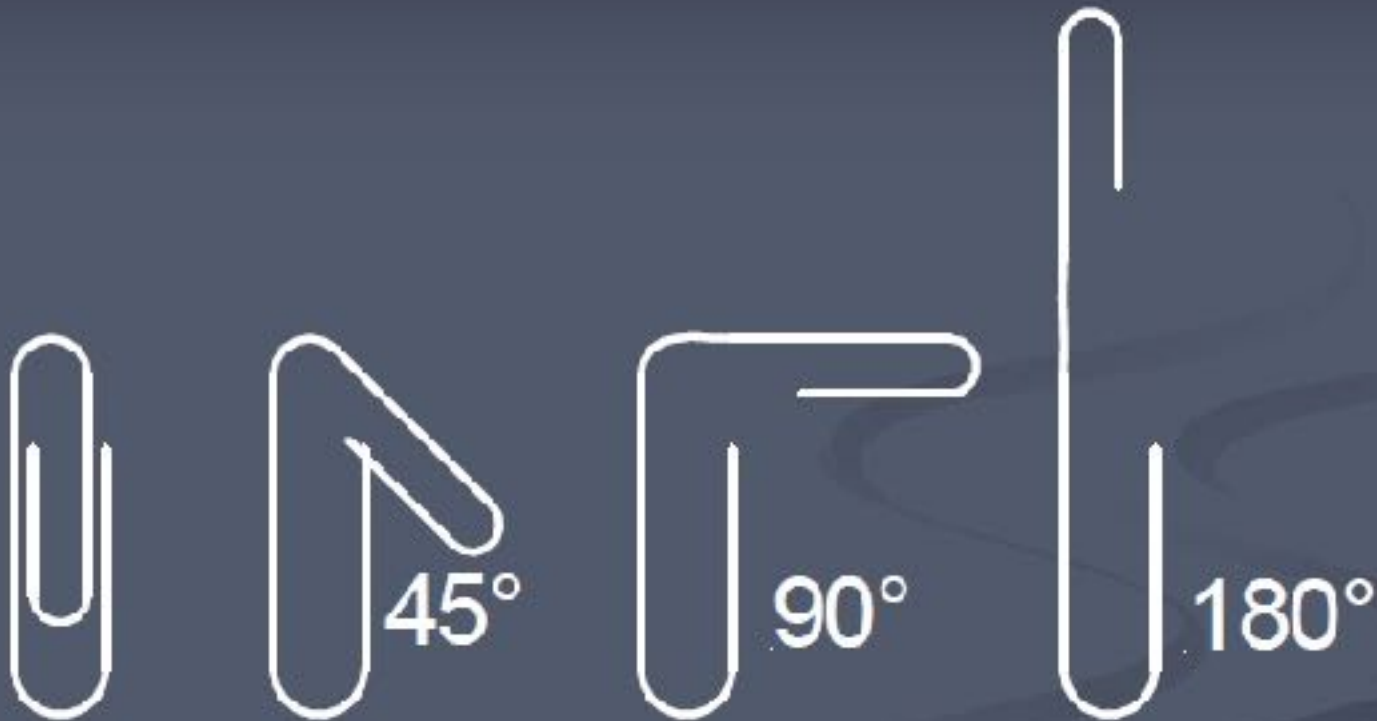
$C_m = 10^M$ factor to adjust laboratory fatigue life for effect of air voids and asphalt volume in asphalt concrete mix

$M = 4.84 * (V_b / (V_b + V_v) - 0.69)$, V_b = asphalt content, % by volume, V_v = volume voids, %

$V_b = P_{ac} / 0.434$ = 11.5 computed from values below in allowable strain repetitions table

$C_m = 0.469$ computed from values below in allowable strain repetitions table

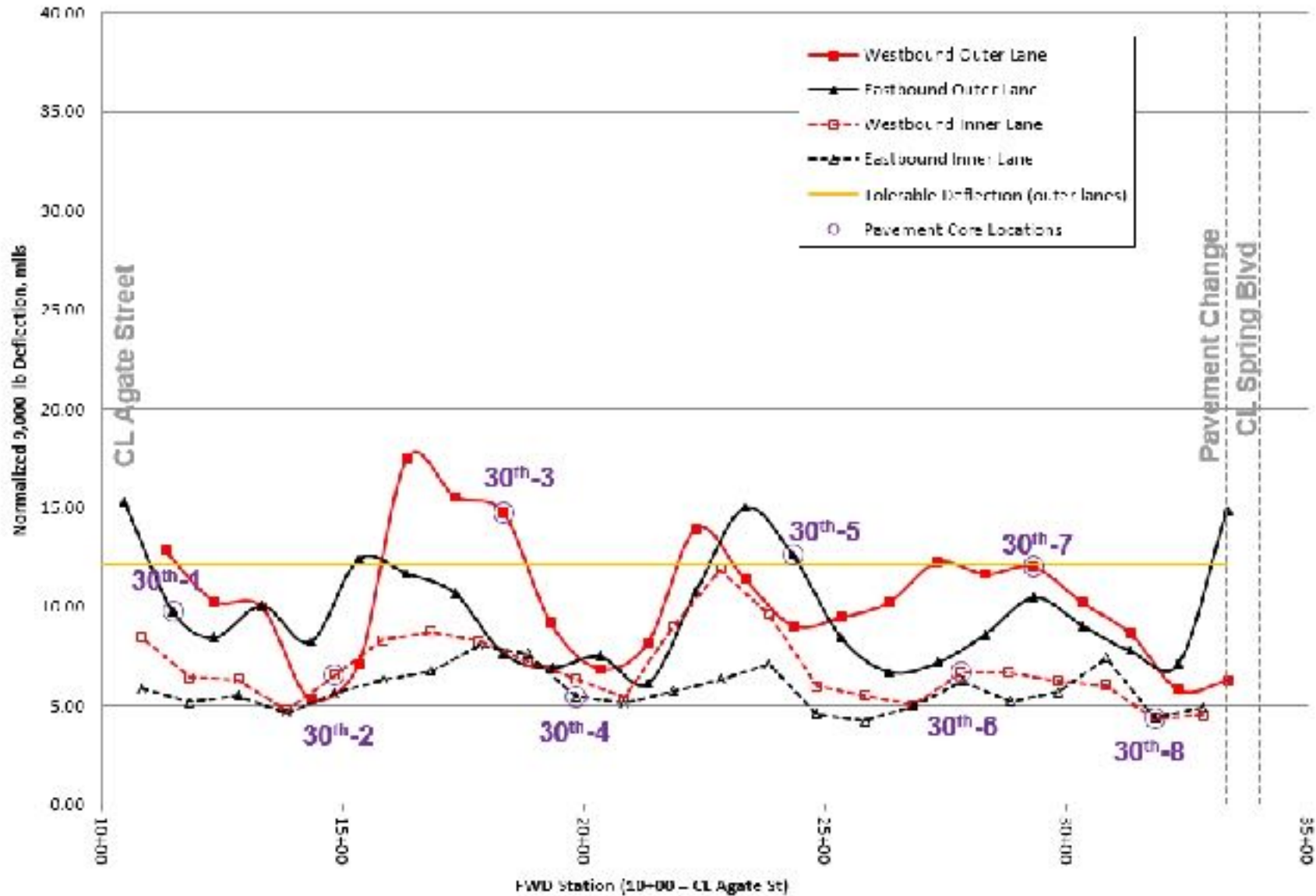
Pavement Fatigue



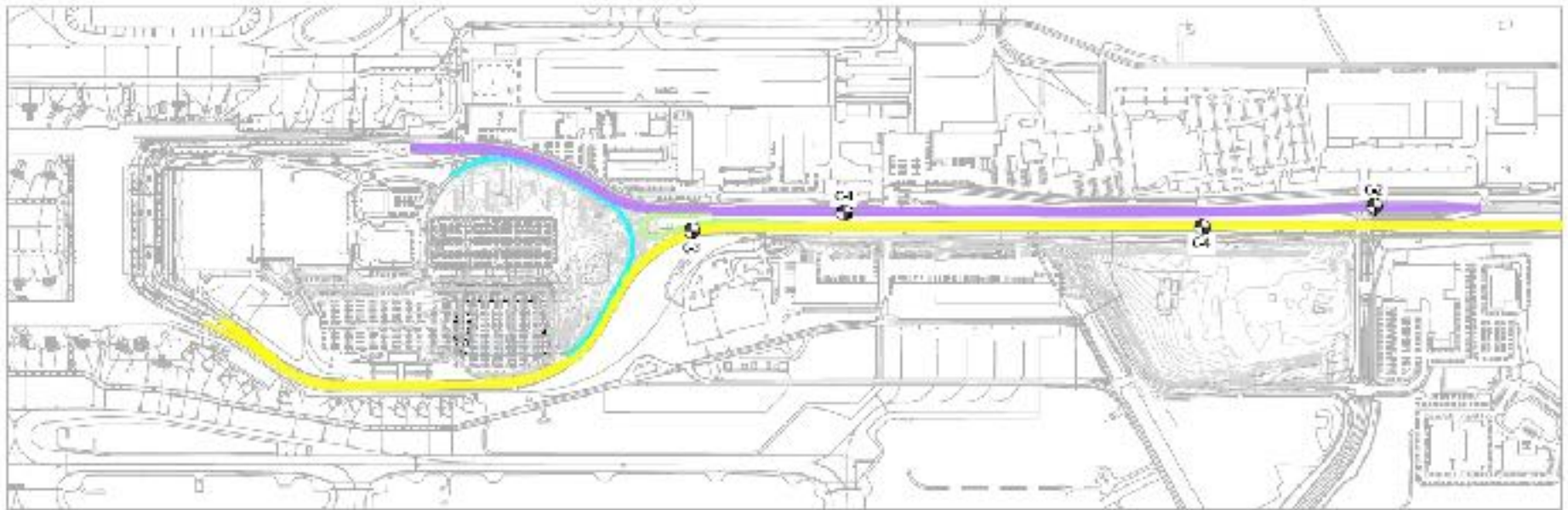
FWD 80/20 Rule

- Use of the FWD as a screening tool
- Network level evaluation





Figure I-B1 - E 30th Avenue: Agate St to Spring Blvd
9-kip Normalized Deflections



Network Level Evaluation



LEGEND

-  Airport Way westbound lanes (3 lanes, 15,700 sq-ft-R)
-  Airport Way westbound lanes (2 to 4 lanes, 21,410 sq-ft-R)
-  Terminal return road (1 lane, 1,770 sq-ft-R)
-  Airport Way eastbound (1 lane, 1,232 sq-ft-R)

 EAST-WEST CORRIDOR STUDY BY GRI
JULY 27, 2014

SITE PLAN: TRAILHEADS EAST OF PORTLAND

North

0 50 100 FT

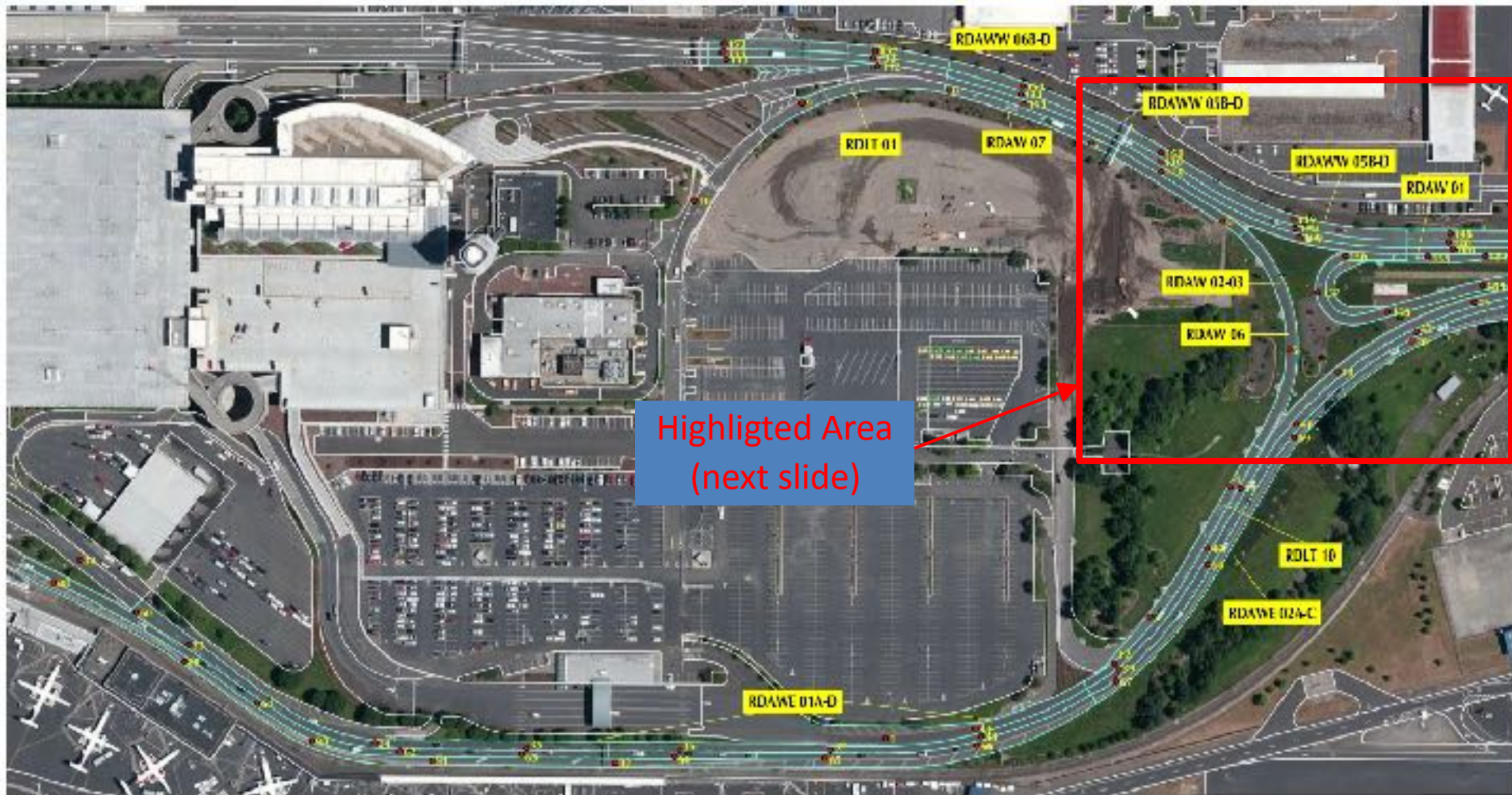
 GRI
GRI CORPORATION
TRAILHEADS EAST OF PORTLAND EVALUATION

SITE PLAN
AIRPORT WAY

JULY 2014 JOB NO. 14021001 REV. 0

 GRI

Network Level Evaluation



Highlighted Area
(next slide)

RTM (AIRPORT) HIGHWAY (DRAFT) 2006-07 JULY 27, 2006
 FWD TESTING COMPLETED BY GRI
 (3/17/2016)

- FWD Test Location (test number is yellow)
- Disturbed FDC Pavement Management Section Boundary
- RDAWE 01 FDC Pavement Management Section ID (East Airport Way, Section 01)

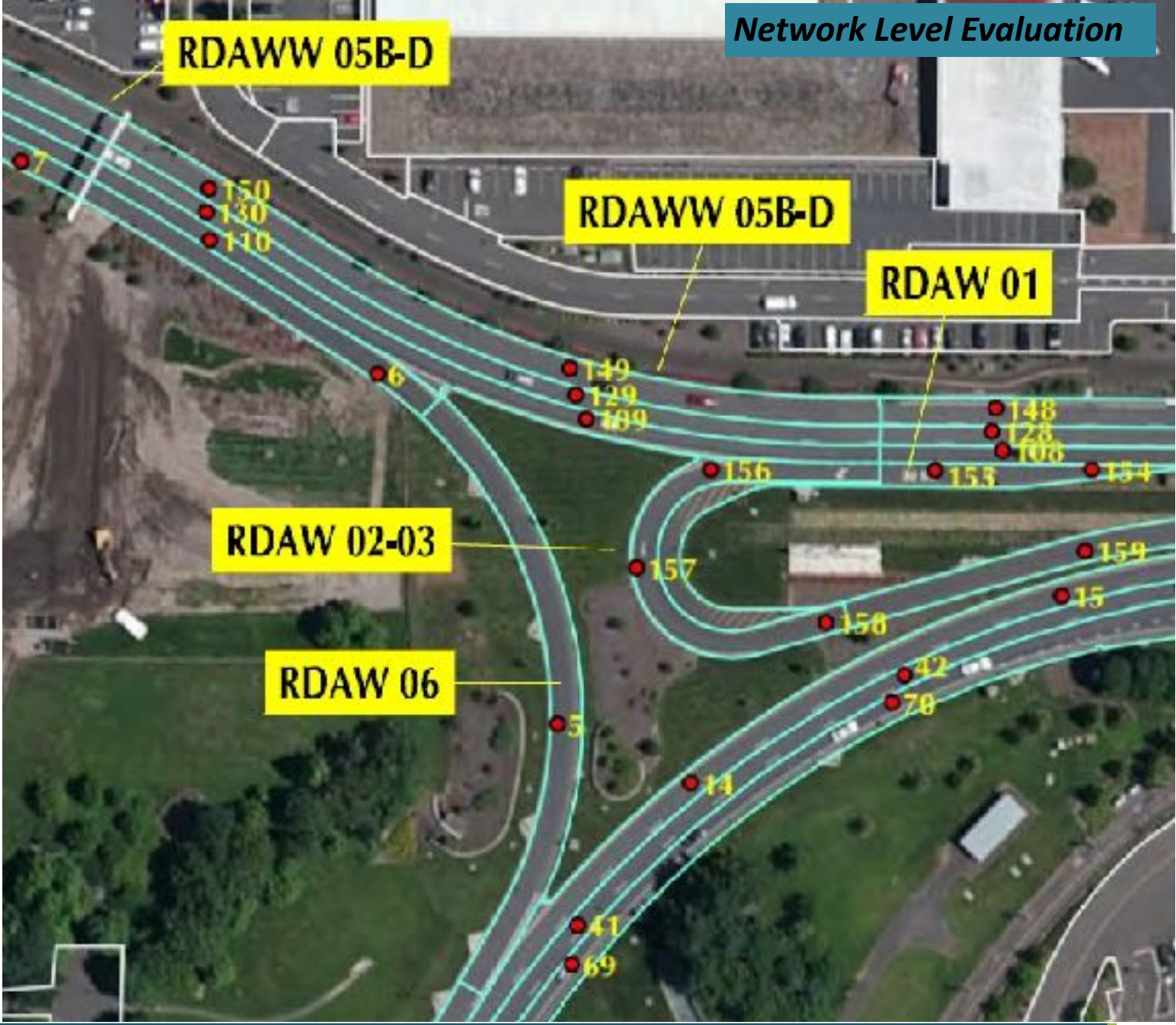


GRI GRI ID# 001100
 A REPORTING & PROSTATE ID NETWORKS CONSULTING

NE AIRPORT WAY & FRONTAGE RD
 FWD TESTING

REV 001 ID# 001100 PAGE 10 OF 10





Network Level Evaluation



PAVEMENT CONDITION INDEX (PCI)

 Good (86 - 100)	 Very Poor (26 - 40)
 Satisfactory (71 - 85)	 Serious (11 - 25)
 Fair (56 - 70)	 Failed (0 - 10)
 Poor (41 - 55)	

DATE: 08/14/2014
PCI CONDITION INDEX SURVEY COMPLETED BY: GRI
DATE: 08/14/2014



GRI PORT OF PORTLAND
A PORTLAND METRO COMMUNITY SERVICES COMPANY

NE AIRPORT WAY & FRONTAGE RD
PCI SURVEY RESULTS

DATE: 08/14/2014

GRI

Network Level Evaluation



BASE RW2 FROM EIR MODIFIED JULY 17, 2018

- Estimated Life: 20+ Years
- Estimated Life: 15-20 Years
- Estimated Life: 10-15 Years
- Estimated Life: 5-10 Years
- Estimated Life: < 5 Years



URS | PORT OF PORTLAND
A STABILITY & FRONTAGE NETWORK LEVEL EVALUATION

NE AIRPORT WAY & FRONTAGE RD
REMAINING PAVEMENT LIFE

DATE: 07/17/18 | SCALE: 1"=100' | SHEET: 101



Network Level Evaluation



AVB\aw2\H05\08\A\03\0001\11\Y21_0816

- | | | | |
|--|-----------------------------|---|----------------------------|
|  | Estimated Life: 20+ Years |  | Estimated Life: 5-10 Years |
|  | Estimated Life: 15-20 Years |  | Estimated Life: < 5 Years |
|  | Estimated Life: 10-15 Years | | |



GRI CITY OF PORTLAND
AIRPORT WAY & FRONTAGE RD NETWORK LEVEL EVALUATION

NE AIRPORT WAY & FRONTAGE RD
REMAINING PAVEMENT LIFE

JUL 2015

CD NO. 044-TWS

PG. 27

Network Level Evaluation

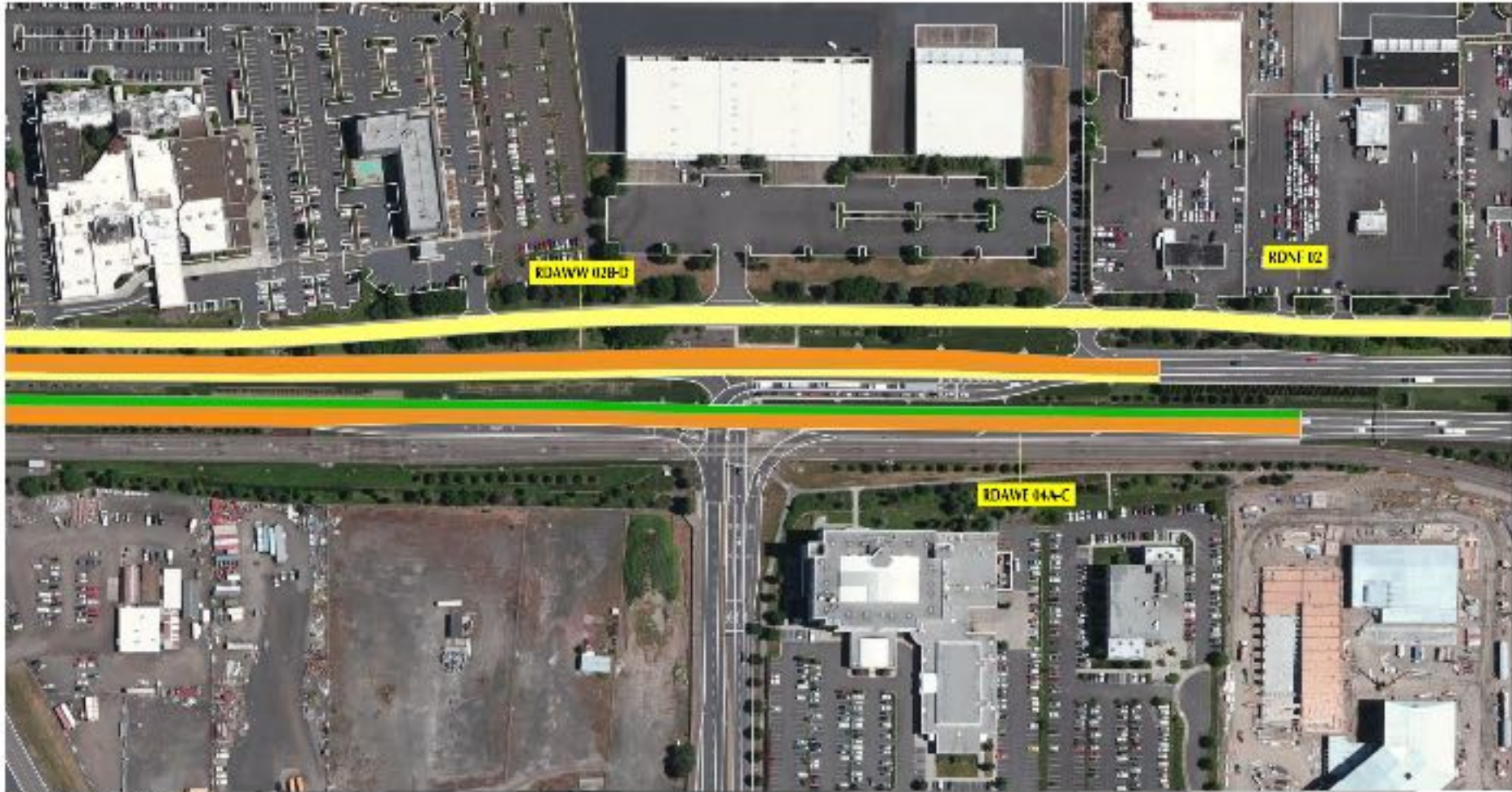


IMAGE FROM GIS ACCESSIBLE JULY 27, 2018

- | | |
|--|---|
| Estimated Life: 10-15 Years | Estimated Life: 5-10 Years |
| Estimated Life: 15-20 Years | Estimated Life: < 5 Years |



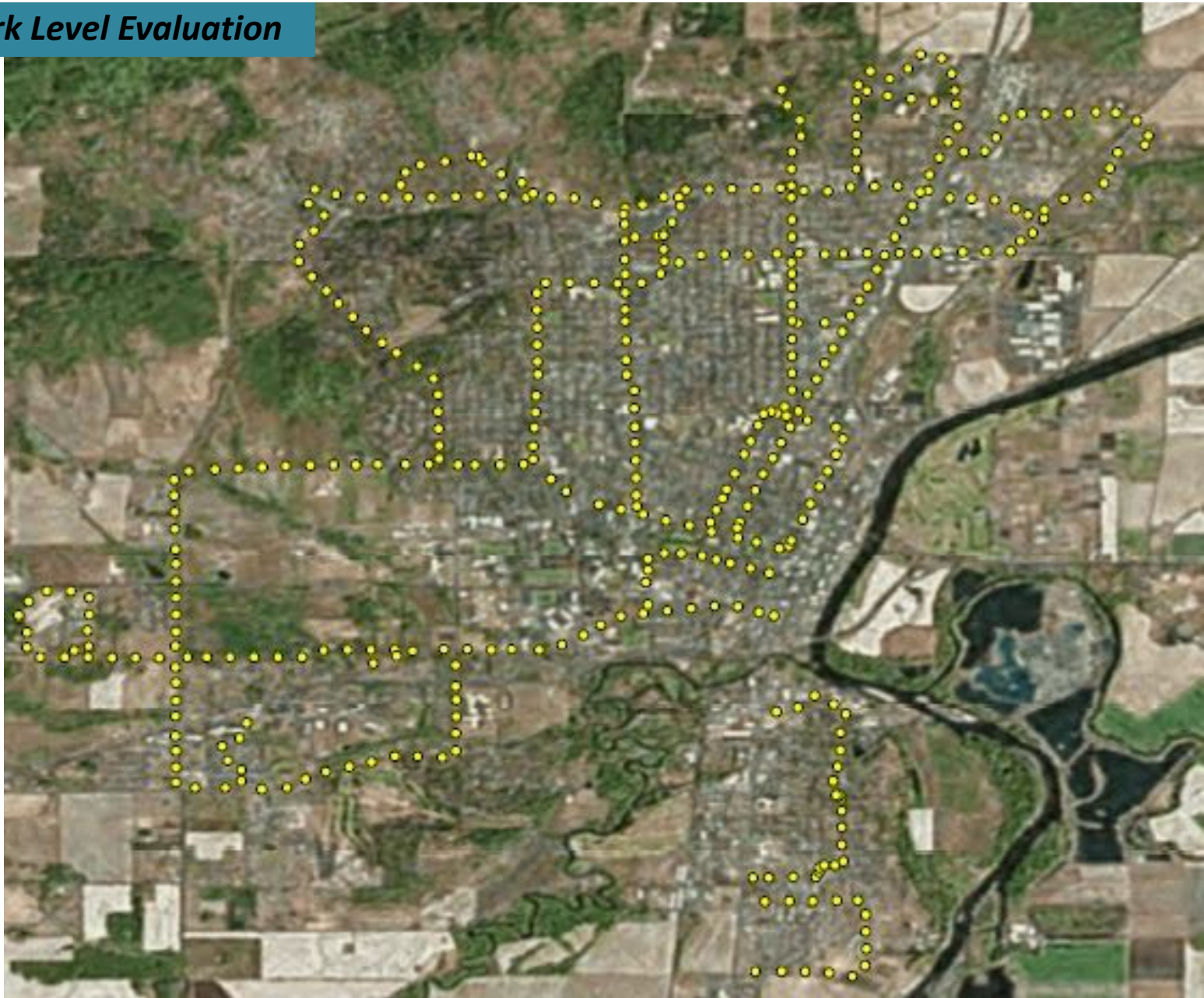
UIC PORT OF IRELAND
 AIRPORT & FRONTAGE ROAD NETWORK EVALUATION

NE AIRPORT WAY & FRONTAGE RD
 REMAINING PAVEMENT LIFE

DATE: 07/27/18 11:58 AM



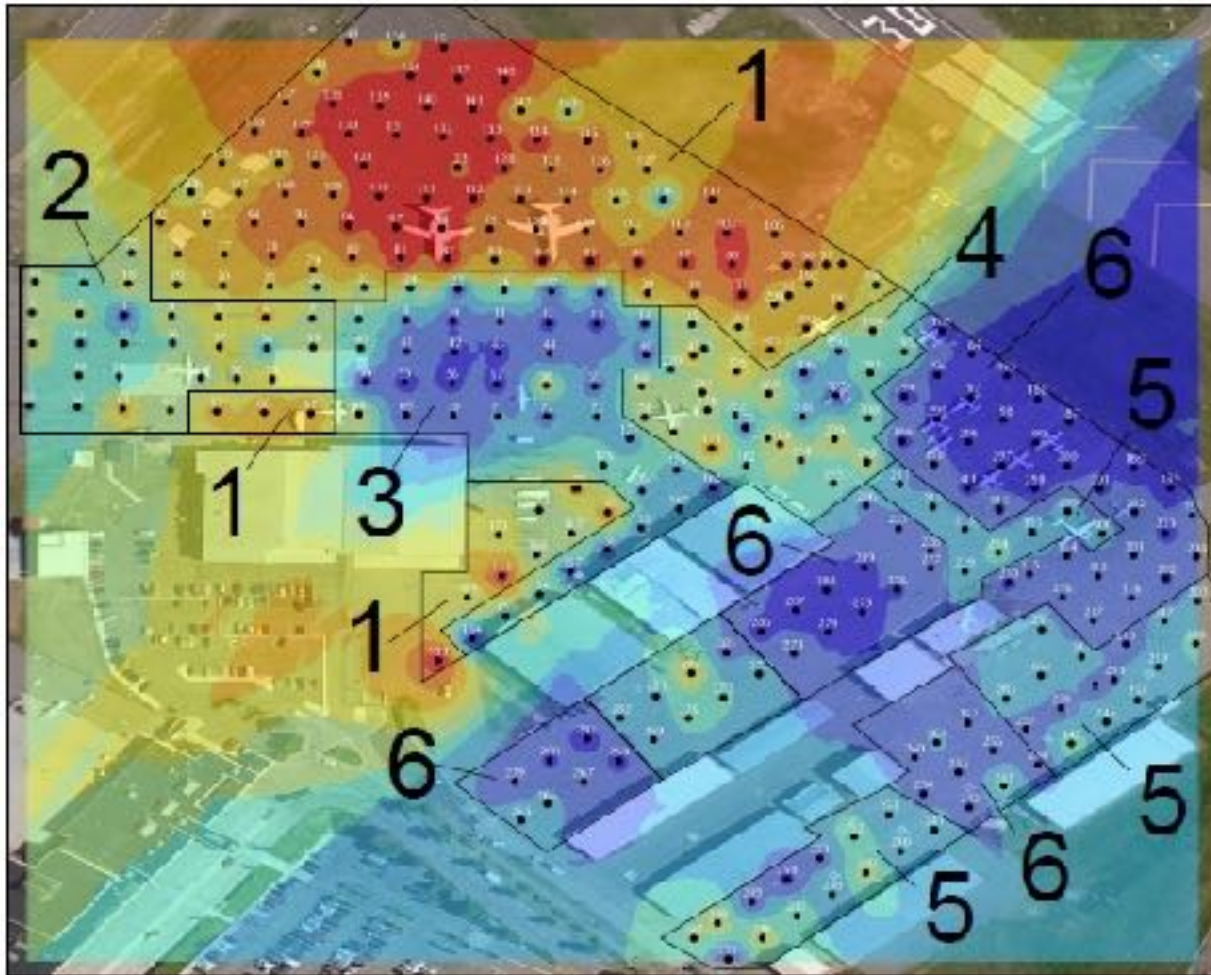
Network Level Evaluation



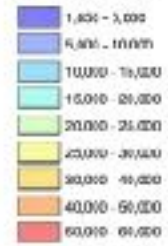
Network Level Evaluation

All Data	PCI	Overlay, in.		
	Category	Average	Average, non zero	85th Percentile
	Very Good	1.15	2.12	2.89
	Good	1.56	2.48	3.73
	Average	1.26	2.41	3.22
	Fair	2.59	3.83	5.78
	Poor	2.31	3.01	4.60
	Very Poor	2.22	2.40	4.60

Backcalculated Subgrade Modulus for Design



BACKCALCULATED SUBGRADE MODULUS, PSI



GRI GRI ENGINEERING, INC.
 LEADER IN INNOVATION AND SERVICE

ANALYSIS UNIT MAP



Questions ?