

# SHRP PG Graded Asphalts



- ⌘ What are they?
- ⌘ What do the numbers mean?
- ⌘ What is the right grade for you?

# SHRP PG Grade Asphalts



- ⌘ Grading system based on climate
- ⌘ Example > PG64-22 (°C)
- ⌘ 64 (147°F) = Average 7 day max. pavement design temp.
- ⌘ High temp. for control of rutting.
- ⌘ -22 (-7°F) = Minimum pavement design temp.
- ⌘ Low temp. for control of thermal cracking.

# SHRP PG Graded Asphalts



- ⌘ Graded in 6 degree increments
- ⌘ Each 6 degree change is called a bump
- ⌘ PG58-28 bumps to PG64 -28 etc.

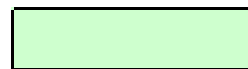
# SHRP PG Graded Asphalts are cumulative



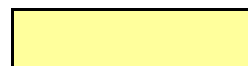
- ⌘ Each grade exceeds the characteristics of the previous grade.
- ⌘ Ex. PG64-22 exceeds PG58-22 in rutting resistance.
- ⌘ EX. PG58-28 exceeds PG 58-22 in thermal cracking resistance.
- ⌘ Ex. PG64-28 exceeds PG58-22 in both rutting resistance and thermal cracking resistance.

# How your PG Grade is made

		High Temperature, °C				
		52	58	64	70	76
Low Temperature, °C	-16	52-16	58-16	64-16	70-16	76-16
	-22	52-22	58-22	64-22	70-22	76-22
	-28	52-28	58-28	64-28	70-28	76-28
	-34	52-34	58-34	64-34	70-34	76-34
	-40	52-40	58-40	64-40	70-40	76-40



= Crude Oil



= High Quality Crude Oil



= Modifier Required

# 90 Degree rule



- ⌘ If you add the top and bottom number and they exceed 90
- ⌘ Your product is most likely modified or upgraded from commodity asphalt
- ⌘  $58-28 = 58+28$  or 86 (Not modified)
- ⌘  $68-28 = 68+28$  or 96 (Likely modified)
- ⌘ If it's a PG+ spec its modified

# PG + Grades



- ⌘ Generally spreads above 90 degrees and they include an Elastic Recovery specification
- ⌘ Idaho – 50% ER
- ⌘ Oregon – 50% ER (also sometimes allows some Non ER 90+ grades)
- ⌘ Washington – 60% ER
- ⌘ Elastic Recovery test stretches the AC sample, cuts it and measures how much the strings return to the original shape.
- ⌘ Some Other states use Ductility specs etc.

# What are the specifications for PG64-28

## ❖ Original Binder tests

- ❖ Flash point > 230 °C
- ❖ Rotational Viscosity < 3.00 @ 135 °C
- ❖ DSR > 1 @ 64 °C

## ❖ RTFO tests

- ❖ Mass loss < 1.0%
- ❖ DSR > 2.2 @ 64 °C
- ❖ ER\* > 60% @ 25 °C

❖ For PG+ in Washington only



# PG64-28 Specifications Cont.



## ❖ PAV tests

❖ DSR < 5000 kPa @ 22 °C

❖ this temp is diff for each grade (see chart)

## ❖ BBR Results

❖ m value > 0.30 @ -18 °C = -28 + 10

❖ Stiffness < 300 @ -18 °C = -28 + 10

# PG Grading Chart Example

Performance Grade	PG 4E			PG 52						PG 58					PG 64						
	34	40	46	10	16	22	28	34	40	46	16	22	28	34	40	10	16	22	28	34	40
Average 7-day Maximum Pavement Design Temperature, °C <sup>a</sup>	< 46			< 52						< 58					< 64						
Minimum Pavement Design Temperature, °C <sup>a</sup>	-34	-40	-46	-10	-16	-22	-28	-34	-40	-46	-16	-22	-28	-34	-40	-10	-16	-22	-28	-34	-40
<b>ORIGINAL BINDER</b>																					
Flash Point Temp, T 48, Minimum, °C <sup>b</sup>	230																				
Viscosity, ASTM D 4402 <sup>b</sup> Maximum, 3 Pa/s, Test Temp, °C	135																				
Dynamic Shear, TP 5 <sup>c</sup> G* (kPa), Minimum, 100 kPa Test Temp @ 10 (12) s, °C	46			52						58					64						
<b>ROLLING THIN FILM OVEN RESIDUE (T 240)</b>																					
Mass Loss, Maximum, percent	1.00																				
Dynamic Shear, TP 5: G* (kPa), Minimum, 220 kPa Test Temp @ 10 (12) s, °C	46			52						58					64						
<b>PRESSURE AGING VESSEL RESIDUE (PP 1)</b>																					
PAV Aging Temperature, °C <sup>a</sup>	90			90						100					100						
Dynamic Shear, TP 5: G* (kPa), Maximum, 5000 kPa Test Temp @ 10 (12) s, °C	10	7	4	25	22	19	16	13	10	7	25	22	19	16	13	31	28	25	22	19	16
Physical Hardening <sup>d</sup>	Report																				
Creep Stiffness, TP 1 Determine the critical cracking temperature as described in PP 42	-24	-30	-36	0	-6	-12	-18	-24	-30	-36	-6	-12	-18	-24	-30	0	-6	-12	-18	-24	-30
Direct Tension, TP 3 Determine the critical cracking temperature as described in PP 42	-24	-30	-36	0	-6	-12	-18	-24	-30	-36	-6	-12	-18	-24	-30	0	-6	-12	-18	-24	-30

# What are we Telling you



## ⌘ Original Binder spec

- ☑ The oil as it is received

## ⌘ Rolling Thin Film Oven Residue

- ☑ How the asphalt behaves after running through the hot plant

## ⌘ Pressure Aging Vessel Residue

- ☑ Physical hardening of the asphalt in the elements

# Supplier Cert.



## Western States Asphalt, Inc.

<b>Certificate of Analysis</b>				
<b>Lot#:</b> SC.7.15.2015.6428.60ER	<b>Specification:</b> WSDOT	<b>Product:</b> PG 64-28 60ER	<b>Sample Date:</b> 7.15.1	
<b>Collection Point:</b> Tank 23			<b>Tested Date:</b> 7.15.1	
<b>BOL #</b>	<b>Key #</b>			
<b>Project Name:</b>				
	<b>Test Parameter</b>	<b>Test Method</b>	<b>Result</b>	<b>Agency Min</b> <b>Agency Ma</b>
	<b>Flash Point, COC</b>	AASHTO T-48		
	Flash Point, COC	3.10.2015	310° C+	230° C+
	<b>Rotational Viscosity @ 135° C</b>	AASHTO T-316		
	Rotational Viscosity		.780	3.000
	<b>Original DSR @ 64° C</b>	AASHTO T-315		
	G*/Sin(delta)		1.70 kPa	1.00 kPa
	<b>Loss on Heating</b>	AASHTO T-240		
	Mass Loss		.4403%	1.000
	<b>RTFO DSR @ 64° C</b>	AASHTO T-315		
	G*/Sin(delta)		3.65 kPa	2.20 kPa
	<b>ELASTIC RECOVERY @ 25° C</b>	AASHTO T-301		
	Elastic Recovery %		75.0%	60%
	<b>PAV (Pressure Aging Vessel)</b>	AASHTO R-28		
	PAV Temp		100° C	
	<b>PAV DSR @ 22° C</b>	AASHTO T-315		
	G*Xsin(delta)		2130 kPa	5000 kPa
	<b>BBR @ -18° C</b>	AASHTO T-313		
	BBR m-Value		.337	0.300
	BBR Stiffness		187	300 MPa

Test data reported herein has been secured by reliable testing procedures. As we have no knowledge of, or control over the conditions that may affect the use of material from which the samples were taken, we assume no responsibility in furnishing this data other than to warrant that they represent reliable measurements of the properties of the sample received and tested.

Certified By: Aaron Payne

Title: Lab Technician

Date: 7.15.2015

# Coming Soon?



- ⌘ A Change to WSDOT PG + Grading System. Maybe other agencies?
- ⌘ No more Elastic Recovery
- ⌘ Multiple Creek Stiffness (MSCR)
- ⌘ No additional Equipment needed
- ⌘ Uses the DSR to determine Mid range characteristics
- ⌘ Currently Being Evaluated

# Agency Test Report

Washington State Department of Transportation  
 State Materials Laboratory  
 PO Box 47365 Olympia WA 98504 / 1655 S. 2nd Ave Tumwater WA 98512  
 Paving Asphalt Test Report

<b>Work Order :</b> 008732 <b>Section :</b> COLFAX TO DRY CREEK - PAVING <b>State Route No :</b> 195 <b>Project Engineer :</b> Simonson, Chad <b>Org Code :</b> 484307	<b>Sample ID :</b> 0000011a764 <b>Lab Number :</b> PG0150252 <b>Bid Item No :</b> 017.03 <b>Date Received :</b> 7/30/2015 <b>Local Agency No :</b>
--	--

<b>Material:</b> PG 64-28 - 9-02.1(4)A - 2014 <b>Supplier:</b> Western States Asphalt <b>Acceptance No:</b> 8 <b>Certificate No :</b> 138868	<b>Manufacturer:</b> Exxon <b>Date Sampled:</b> 7/20/2015
---	--

ORIGINAL PROPERTIES :	RESULTS :	SPECIFICATION :
ROTATIONAL VISCOSITY @ 135° C, Pas	0.8	3 MAXIMUM
DYNAMIC SHEAR @ 64°C, kPa	1.64	1.00 MINIMUM
C.O.C. FLASH POINT, C		
RTFO AGED PROPERTIES :		
MASS LOSS AFTER RTFO, %	0.14	1.00 MAXIMUM
ELASTIC RECOVERY, %	72	60 MINIMUM
DYNAMIC SHEAR @ 64°C, kPa	3.51	2.20 MINIMUM
MSCR @ 3.2 kpa @ 64°C	22.70	REPORT ONLY
Jnr @ 3.2 kpa @ 64°C	1.82	REPORT ONLY
DIFFERENCE Jnr, %	55.67	REPORT ONLY
MSCR and Jnr testing performed at PG base grade temperature		
PAV AGED PROPERTIES :		
PAV AGING TEMPERATURE 100° C		
DYNAMIC SHEAR @ 22°C, kPa	2700	5000 MAXIMUM
BENDING BEAM RHEOMETER @ -18°C		
"S", MPa	156	300 MAXIMUM
"m"	0.326	0.300 MINIMUM

**Result Code:** Meets Specifications

**Remarks :**

Kurt R. Williams, P.E.  
 State Materials Engineer  
 Steven J. Davis  
 Bituminous Materials Engineer  
 Date : 8/7/2015  
 Phone : (360) 709-5424

**Billing Code**

- T119 - 3
- T120 - 1
- T121 - 1
- T124 - 1
- T130 - 1
- T131 - 1
- T117 - 1


# Grades might look like this

## WSDOT Eastside Currently

- ⌘ 64-28 Non ER
- ⌘ 64-28 60ER Base Grade
- ⌘ 70-28 60ER = 1 Bump
- ⌘ 76-28 60ER = 2 Bumps

## WSDOT Eastside under MSCR

- ⌘ 64-28 S
- ⌘ 64-28 H
- ⌘ 64-28 V
- ⌘ 64-28 E
- ⌘ Subject to Change



How do you determine the right grade for your project?



# "LTPPBind"



- ⌘ Windows based software for selection of performance graded asphalt binders.
- ⌘ Available free from the Federal Highway Administration.
- ⌘ <http://tfhrc.gov/>

# "LTTPBind"



⌘ Use this software to determine correct grade

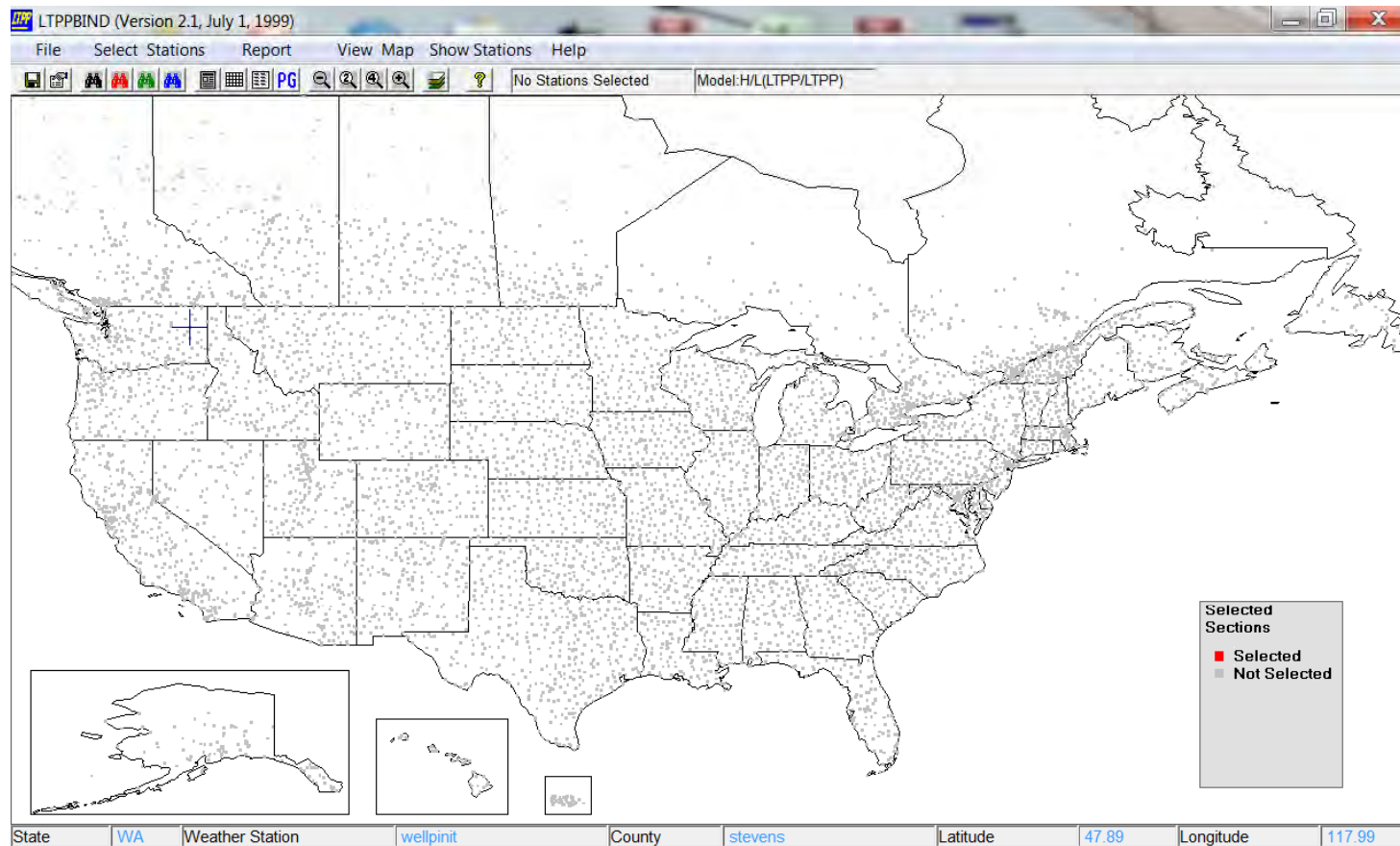
☑ Determine correct base grade

☑ Determine ESAL's

☑ Determine Traffic Speed

LTTP Bind will determine the proper bumps needed to accommodate your traffic conditions.

# Find your nearest weather station



# You can compare the three closest Stations

Three Closest Weather Stations For Latitude/Longitude= 48.52986/117.8756

General	A=2 km	B=3 km	C=32 km
State	WA	WA	WA
Station ID	0451650	0451630	0451395
County/District	stevens	stevens	stevens
Weather Station	colville ap	colville	chewelah
Elevation, m	575	505	509
Latitude, Longitude	48.55 , 117.88	48.55 , 117.90	48.28 , 117.72
Last Year Data Available	1986	1952	1996

Air Temperature	Mean ( Std, N )	Mean ( Std, N )	Mean ( Std, N )
Average 7-day High Temp.	35.1 (1.8, 27)	35.1 (1.4, 34)	35.5 (2.0, 41)
Low Temperature	-23.5 (5.6, 30)	-24.5 (6.0, 33)	-25.0 (5.7, 38)
Low Temperature Drop	15.4 (3.1, 28)	16.6 (3.9, 32)	20.7 (4.7, 36)
Degree Days Above 30 C	121 (55, 27)	139 (53, 34)	154 (71, 41)

Pavement Temp. and PG	High Low Rel.	High Low Rel.	High Low Rel.
=50% Rel. Pavement Temp.	50.8 -19.2 (50,50)	50.8 -19.9 (50,50)	51.2 -20.1 (50,50)
>50% Rel. PG (High, Low Rel.)	52 -22 (64,73)	52 -22 (64,67)	52 -22 (59,65)
	58 -22 (98,73)	58 -22 (98,67)	58 -22 (97,65)
	58 -28 (98,97)	58 -28 (98,95)	58 -28 (97,95)
	58 -34 (98,98)	58 -34 (98,98)	58 -34 (97,98)
			64 -34 (98,98)

Close PG Chart Print Save Help

# What's the base grade?

## 98% Reliability

PG Binder Selection

Data for 'COLVILLE AP' Weather Station

Latitude, Degree: 48.55

Design Air Temperature, Degree C: HIGH 35.1, LOW -23.5

Air Temperature Standard Deviation, C: 1.8, 5.6

Other Inputs

Desired Reliability, %: 98

Depth (Pvt. surface to top of layer, mm): 0

Traffic Load, Million ESAL: 0

Traffic Speed: Fast

Traffic Adjustment

None

SHRP

KMC

User Defined

View / Modify

Pavement Temperature and PG	HIGH	LOW
Design Air Temperature	35.1	-23.5
Design Pavement Temperature	57.6	-28.5
Adjustment for Traffic Speed	+ 0	
Adjustment for Traffic Loading	+ 0	
Adjusted Pavement Temperature	57.6	-28.5
Selected Binder Grade	58	-34

Close PG Chart Print Save Help

# Lets increase the traffic load

**PG Binder Selection**

Data for 'COLVILLE AP' Weather Station

Latitude, Degree: 48.55

Design Air Temperature, Degree C: HIGH 35.1, LOW -23.5

Air Temperature Standard Deviation, C: 1.8, 5.6

Other Inputs

Desired Reliability, %: 98

Depth (Pvt. surface to top of layer, mm): 0

Traffic Load, Million ESAL: 3

Traffic Speed: Fast

Traffic Adjustment

None

SHRP

KMC

User Defined

View / Modify

Pavement Temperature and PG	HIGH	LOW
Design Air Temperature	35.1	-23.5
Design Pavement Temperature	57.6	-28.5
Adjustment for Traffic Speed	+ 6	
Adjustment for Traffic Loading	+ 0	
Adjusted Pavement Temperature	63.6	-28.5
Selected Binder Grade	64	-34

Close PG Chart Print Save Help

# Lets slow down the traffic

PG Binder Selection

Data for 'COLVILLE AP' Weather Station

Latitude, Degree: 48.55

Design Air Temperature, Degree C: HIGH 35.1, LOW -23.5

Air Temperature Standard Deviation, C: 1.8, 5.6

Other Inputs

Desired Reliability, %: 98

Depth (Pvt. surface to top of layer, mm): 0

Traffic Load, Million ESAL: 3

Traffic Speed: Slow

Traffic Adjustment

None

SHRP

KMC

User Defined

View / Modify

Pavement Temperature and PG	HIGH	LOW
Design Air Temperature	35.1	-23.5
Design Pavement Temperature	57.6	-28.5
Adjustment for Traffic Speed	+ 6	
Adjustment for Traffic Loading	+ 6	
Adjusted Pavement Temperature	69.6	-28.5
Selected Binder Grade	70	-34

Close PG Chart Print Save Help

# Lets really stress the pavement with stopped traffic

PG Binder Selection

Data for 'COLVILLE AP' Weather Station

Latitude, Degree: 48.55

Design Air Temperature, Degree C: HIGH 35.1, LOW -23.5

Air Temperature Standard Deviation, C: 1.8, 5.6

Other Inputs

Desired Reliability, %: 98

Depth (Pvt. surface to top of layer, mm): 0

Traffic Load, Million ESAL: 3

Traffic Speed: Standing

Traffic Adjustment

None

SHRP

KMC

User Defined

View / Modify

Pavement Temperature and PG	HIGH	LOW
Design Air Temperature	35.1	-23.5
Design Pavement Temperature	57.6	-28.5
Adjustment for Traffic Speed	+ 6	
Adjustment for Traffic Loading	+ 12	
Adjusted Pavement Temperature	75.6	-28.5
Selected Binder Grade	76	-34

Close PG Chart Print Save Help



# Other Mix Design Concerns



- ⌘ Don't forget that not only is the proper PG grade important in your mix design but also the number of gyrations used in the design.
- ⌘ Higher Traffic counts require more gyrations to be used in the design.
  - ☒ Too few gyrations used in a high traffic design can result in an overly rich mix susceptible to excessive post compaction, flushing and rutting.
  - ☒ Conversely, too many gyrations used in a low traffic design can result in a dry mix susceptible to raveling.

# Caution



- ⌘ Higher grades can be inserted into a mix and outperform lesser grades, however,
- ⌘ Just because a grade exceeds another does not mean you can just replace it without a new mix design.
- ⌘ Each grade has different viscosities and formulas that can affect mix design.
- ⌘ You may want your supplier to have a separate mix design to ensure quality.

# Old vs. New



⌘ PBA-6	similar to	PG 64-28+
⌘ AC20	similar to	PG 64-22
⌘ AR4000W	similar to	PG 58/64-22
⌘ AC10	similar to	PG 58-28

# Old vs. New



- ⌘ Use what works for you.
- ⌘ If you had success with a grade in the past you don't have to change. Use the PG grade similar to your old grade.
- ⌘ If you did not, look at the options that SHRP PG grading and your supplier have available to give you.
- ⌘ PG grading is only a tool past history is still very important.



⌘ Thanks

⌘ Questions ?

⌘ Stephen Van De Bogert

⌘ Western States Asphalt LLC

⌘ 509-994-2462