

# **INTRODUCTION TO ASPHALT MATERIALS FOR MANAGERS AND OWNERS**

**ASPHALT PAVEMENT ASSOCIATION  
OF OREGON**

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# BACKGROUND

- **Flexible Pavements**
- **Part of a Layered/Composite System**
- **Elastic Behavior**



# BACKGROUND

- **National Effort to Improve Asphalt Pavements**
- **Strategic Highway Research Program (SHRP)**
- **SuperPave™ Mix Design System**
- **Rely on good Mix Design Practices and Field Management**



# WHAT MAKES GOOD MIX?

- **Mixes with the best performance have *by volume*:**
  - **96% Aggregate and Oil**
  - **4% Air Voids**



# WHAT MAKES GOOD MIX?

- **Rollers usually get pavement to about 8% air voids, 92% aggregate and oil.**



# WHAT MAKES GOOD MIX?

**Goal: design an asphalt mix that will stabilize at 4% air voids after one to three years of secondary compaction.**

**Secondary Compaction: 4% more compaction**



# MIXTURE TYPE SELECTION

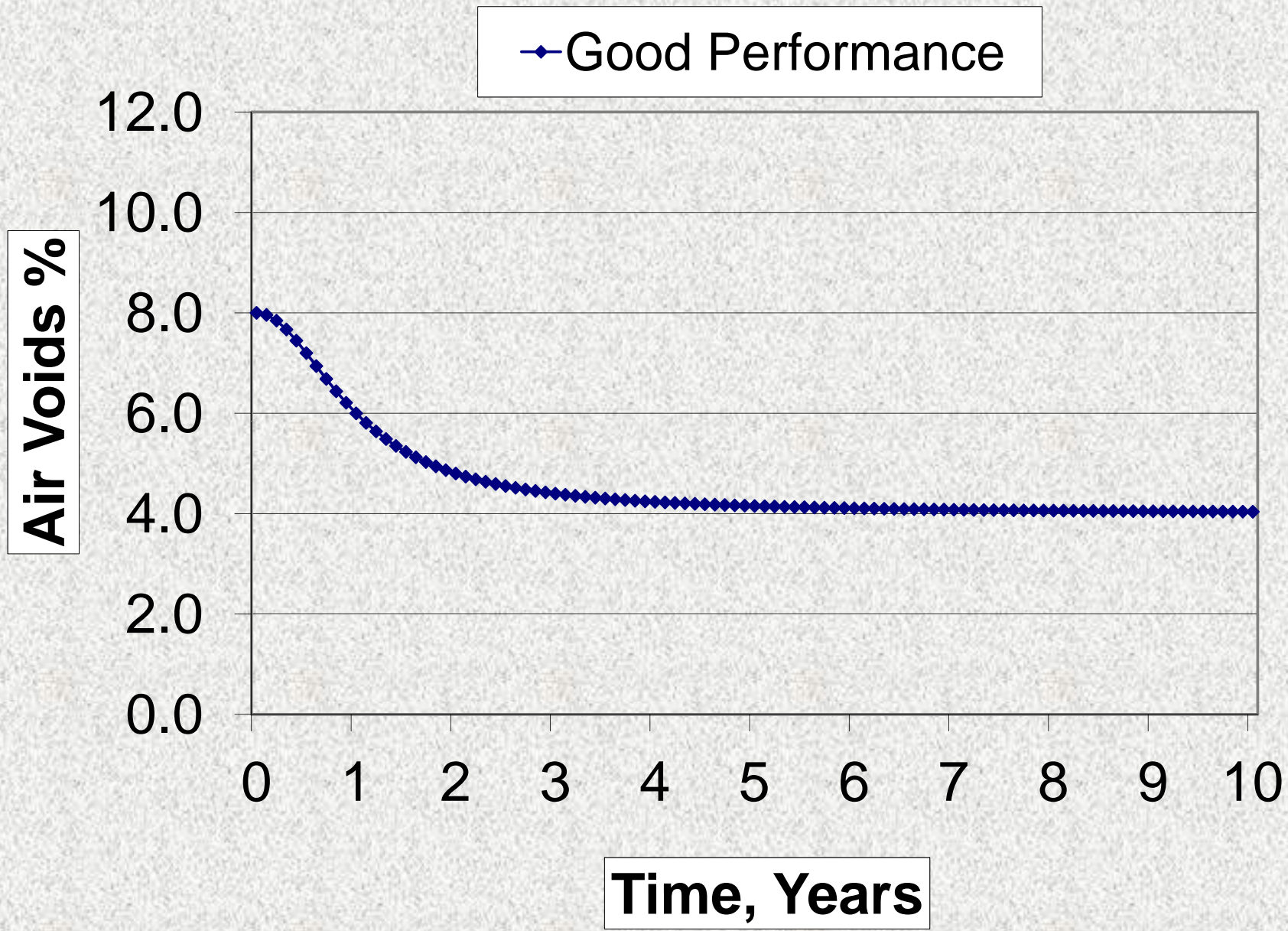
Level	Traffic Type	20 Yr EALs
1	Low traffic, few or no trucks, 1 / day	0 - 30,000
2	Moderate truck traffic, < 140 /day	30,000 - 1 million
3	Heavy truck traffic	1 - 10 million
4	Very heavy truck traffic	> 10 million

# WHAT MAKES GOOD MIX?

*If the pavement receives significantly more or significantly less secondary compaction than the design amount, then performance of the pavement will be negatively impacted.*



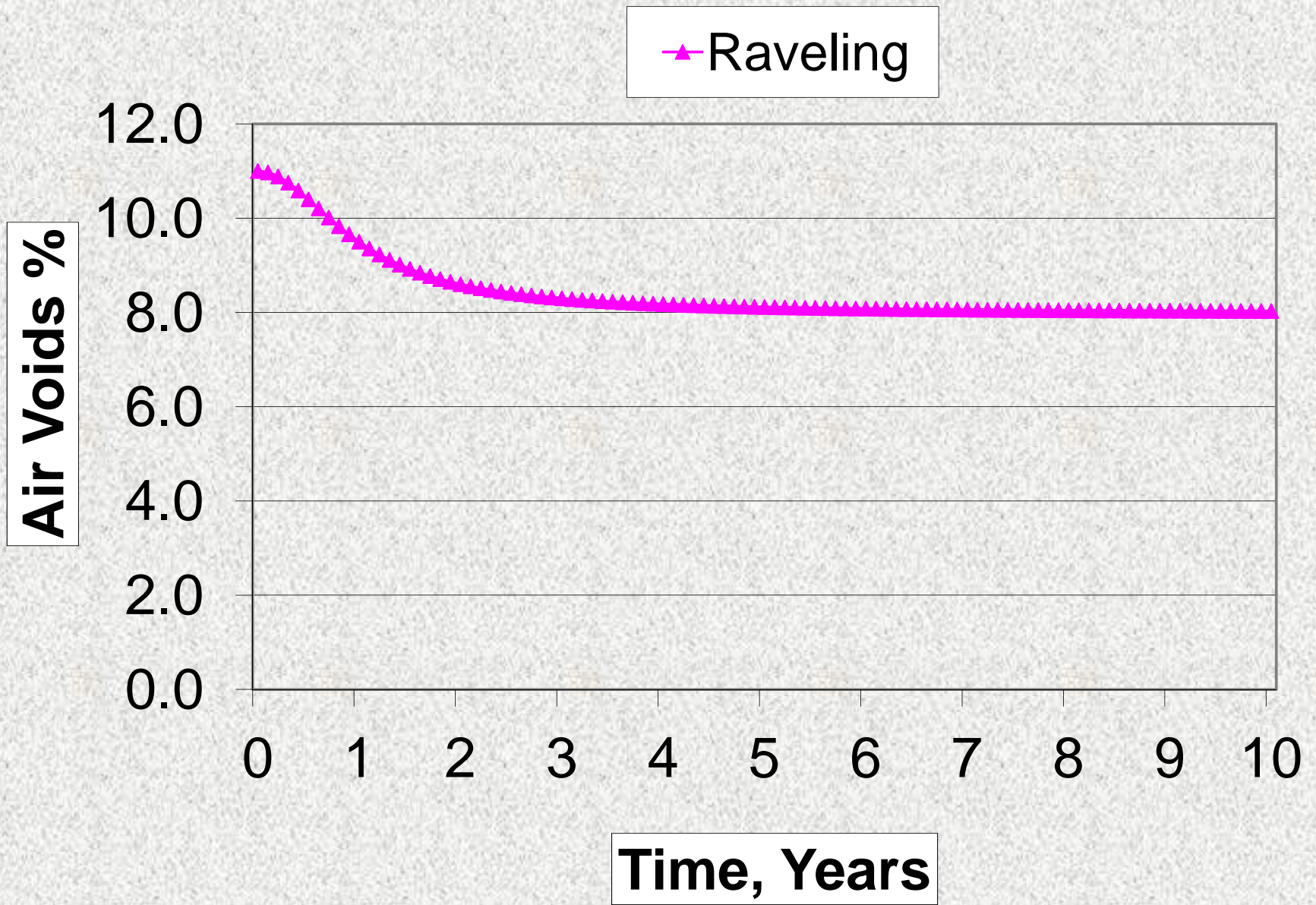




# WHAT MAKES GOOD MIX?

Pavements that don't properly densify after construction are much more prone to "raveling" (loss of fine surface matrix).



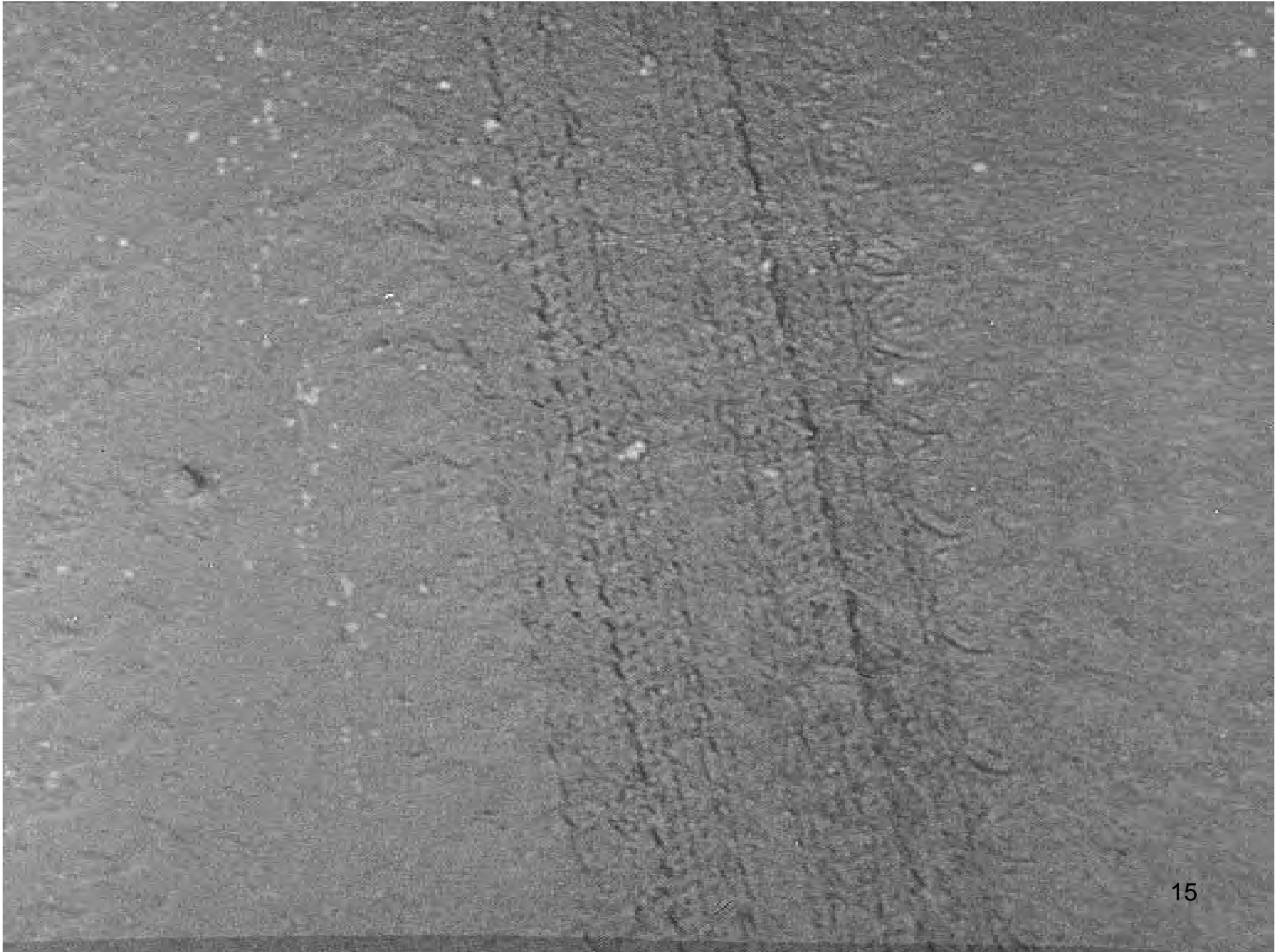


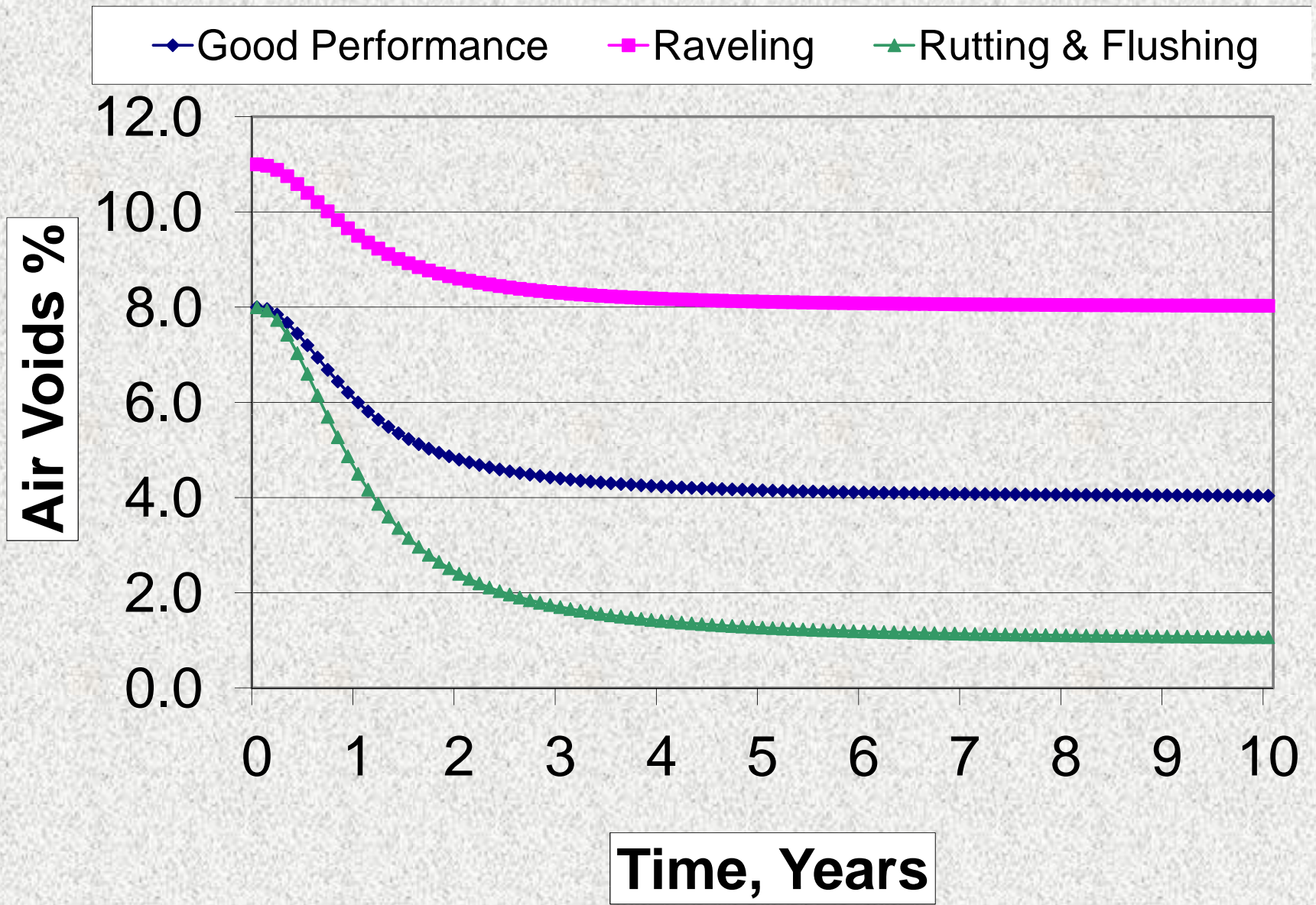
# **The Reverse Problem – Too Much Secondary Compaction**

**A classic case for creating a pavement that ruts or flushes is to use a mixture designed for a low traffic application such as a parking lot mix and putting it on a high volume roadway such as an “arterial” or “truck route.”**











# Modeling Secondary Compaction

- **Correctly quantifying energy to model secondary compaction is *not easy***
- **Each “Mix Design” is tailored to level of anticipated traffic**



## **SUPERPAVE™ GYRATORY – ATM**

- **Level establishes the amount of energy imparted to lab specimens in mix design process.**
- **Energy comes from the gyrations used in the gyratory compactor.**

# **SUPERPAVE™ GYRATORY - ATM**

- **Use same mixing and placement temps as construction**
- **Cure for two hours at elevated temperature to allow absorption to occur.**



# **INTRODUCTION TO MIX DESIGN**

## **4 Step Process**

- **Material Selection**
- **Select Aggregate Blend (Stage 1)**
  - **Three different gradations**
- **Select Asphalt Binder Content (Stage 2)**
  - **Four different asphalt contents**
- **Performance Testing (Stage 3)**
  - **Two performance tests**

# **MATERIAL SELECTION**

- **Aggregate**
- **Recycled Asphalt Materials (Optional)**
  - **RAP (Asphalt Pavement)**
  - **RAS (Asphalt Shingles)**
- **Asphalt Cement (binder)**
- **Additives (lime, anti-strip, etc.)**

# AGGREGATE MATERIALS

- **Skeleton (94 to 95% of mix)**
  - **Withstand loads**
  - **Resist damage from traffic and environment**
- **Source Dependent Properties**



# AGGREGATE MATERIALS

## Source Review Testing

- **In Oregon, Aggregate Quality Testing is done prior to Developing a Source**

# AGGREGATE MATERIALS

## Product Compliance Testing

- **After Aggregate Production begins**
- **Required for each Stockpile**
- **Required prior to JMF Approval**





# AGGREGATE MATERIALS

## Separated Sizes (Aggregate Stockpiles)

**Coarse Stockpiles: + No. 4 sieve**

**Fine Stockpiles: - No. 4 sieve**



# Particle Shape

- **Fractured Particles**
- **Flat and Elongated**
- **Blend Sand**



# Passing No. 200 Material

- **No. 200** refers to **200 openings per lineal inch of screen**
- **“Dust”** has large impact on mix properties
- **Mitigating stockpiles with excessive dust:**
  - **Blend with cleaner piles**
  - **Wash**
- **Waste Baghouse Dust**



# Quality Level Analysis

- **Statistical Summary referred to as Quality Levels (QL)**
- **Designers use QL's to determine JMF target blend**

# RECYCLED ASPHALT MATERIALS

## RAP

- Asphalt Content
- Gradation
- Based on testing by Ignition Method



# **RECYCLED ASPHALT MATERIALS**

- **Amount of RAP Allowed:**
  - **30% for dense-graded**
  - **20% for Level 4 dense-graded Wearing Courses**
- **Practical Limits:**
  - **RAP Availability**
  - **Plant Capacity/EPA Emissions Requirements**
- **Benefits:**
  - **Improves rut and stripping resistance**
  - **Sustainable practice**
  - **Reduced cost**

# **RECYCLED ASPHALT MATERIALS**

## **RAS**

- **Two Categories:**
  - **Manufacturing Waste (MW)**
  - **Tear Offs (TO)**
- **Both Categories require shredding**
- **TO-RAS may require additional steps to remove fasteners and debris**
- **Special Considerations**
  - **Stiffer aged asphalts**
  - **High Passing No. 200 (dust)**
  - **~ 5% RAS allowed**

# SELECTION OF ASPHALT BINDER

- **PG Grading system (PG 64 – 22)**





# **SELECTING THE BINDER GRADE**

- **Climate – High and Low Pavement Temp**
- **Volume of traffic**
- **Speed of traffic**
- **RAP content**

# SPECIFIC GRAVITY AND VOLUMETRICS

## INTRODUCTION TO SPECIFIC GRAVITY

- Density Compared to  
H<sub>2</sub>O

$$\text{Density} = \frac{\text{Mass}}{\text{Volume}}$$



# INTRO TO SPECIFIC GRAVITY

- Volumes of even sided Geometric Objects are easy to Measure
- Volumes of Irregular Objects (HMAC mix) are not so easy to Measure
- Specific Gravity is an easy means to measure the Volume of Irregular Objects

# MEASURING SPECIFIC GRAVITY

## Definition:

*Specific Gravity* – the ratio of the weight in air of a unit volume of a material at a stated temperature to the weight of an equal volume of gas-free distilled water at a stated temperature.

# FIVE FUNDAMENTAL GRAVITIES

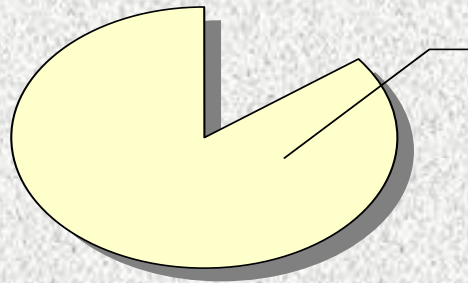
- Liquid Asphalt Sp. Gr.,  $G_b$
- Bulk Sp. Gr. of Aggregate,  $G_{sb}$
- Effective Sp. Gr. of Aggregate,  $G_{se}$
- Maximum Sp. Gr. of Mixture,  $G_{mm}$
- Bulk Sp. Gr. of Compacted Mix,  $G_{mb}$

# BULK SP. GR. OF AGGREGATE

$$G_{sb} = \frac{P_1 + P_2 + \dots + P_n}{\frac{P_1}{G_1} + \frac{P_2}{G_2} + \dots + \frac{P_n}{G_n}}$$

- Includes pore space in the aggregates
- AASHTO T 85 for Coarse Aggregate
- AASHTO T 84 for Fine Aggregate (weakest link)
- Typical values: 2.400 to 2.800

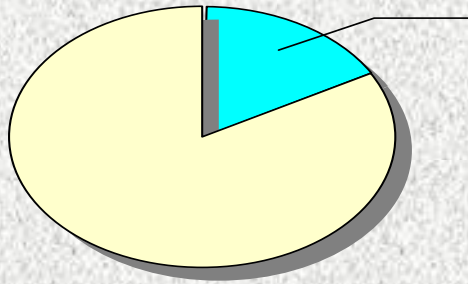
## COMPARISON OF AGGREGATE GRAVITIES



Volume of Dry Aggregate

*(Smallest Volume)*

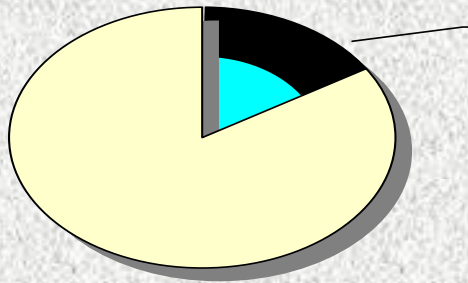
Used to Determine Apparent  
Specific Gravity,  $G_{sa}$



Volume of Water Permeable Voids  
plus  
Volume of Dry Aggregate

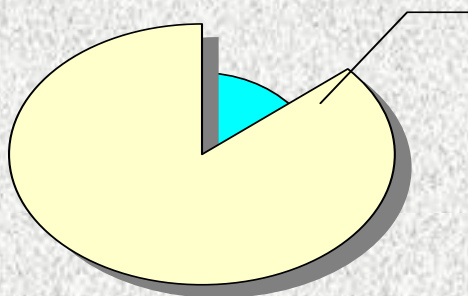
*(Largest Volume)*

Used to Determine Bulk Specific  
Gravity,  $G_{sb}$



Volume of Asphalt Permeable  
Voids

*(Note: Is less than the  
Volume of Water Permeable Voids)*



Volume of Water Permeable  
Voids less  
Volume of Asphalt Permeable  
Voids plus  
Volume of Dry Aggregate

*(Intermediate Volume)*

Used to Determine the Effective  
Specific Gravity,  $G_{se}$

# MAX SPECIFIC GRAVITY OF MIXTURE

- **“Rice” Test**
- **Performed on loose mix**
  - **Tends to trap air bubbles**
  - **Vacuum**





# BULK SPECIFIC GRAVITY OF COMPACTED MIXTURE

- Lab gyratory sample
- Mix with air voids
- Weight in water method



# INTRO TO VOLUMETRICS

- **Volume occupied by each constituent:**
  - **Aggregate**
  - **Asphalt Cement**
  - **Air Voids**
- **Based on specific gravities and masses**



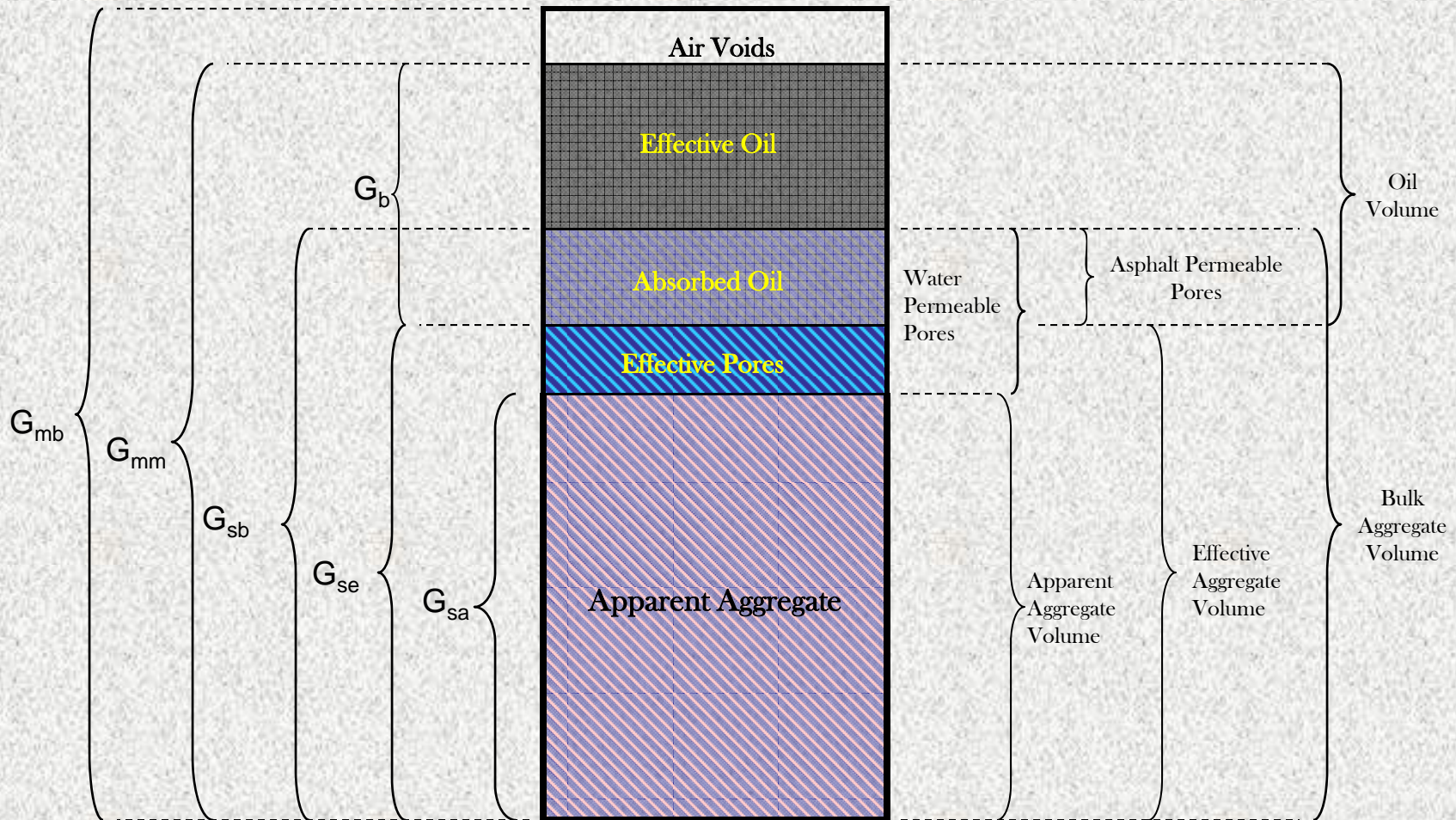
# Percent by Volume

$V_a$  = Air Voids

VMA = Voids in Mineral Aggregate

VFA = Voids Filled with Asphalt

# HMAC Constituent Diagram



## **Definitions:**

**Voids in Mineral Aggregate, VMA: volume of void space between the aggregate particles of a compacted paving mixture that includes air voids and effective asphalt content.**

**Expressed as a percent of the total volume of the sample.**

## **Fundamental Relationship:**

$$VMA = V_a + V_{be}$$

- **Typical values: 13.0 to 17.0%**

# SUMMARY OF VOLUMETRICS

- **Specific Gravity is used to measure the density of irregular shaped objects**
- **Volumetrics for Mix Design and QC come from specific gravities**
- **Monitor and manage the five specific gravities**



# DENSE-GRADED MIX DESIGNS

**Stage 1: Selection and testing of three trial aggregate blends**

**Stage 2: Selection and testing of the JMF blend with four asphalt contents**

**Stage 3: Selection and testing of the JMF blend and asphalt content for rut susceptibility and moisture sensitivity (stripping)**





# STAGE 1: AGGREGATE GRADATION

- **CMDT seeks a blend which meets the design criteria**
- **Gradation Controls:**
  - **VMA**
  - **P200 to Effective Ratio**
- **Historically used a single “best guess” blend**
- **SuperPave™ requires three blends**



# **IDEAL GRADATION TARGETS**

- **The ideal targets for various mix types have been around for many years**
- **The street term for ideal targets is “Golden Gradations”**

# SELECTION OF TRIAL GRADATIONS

- **Review Historical Data:**
  - **Product Compliance Data**
  - **Existing Mix Designs**
  
- **Look for:**
  - **VMA problems**
  - **P200 problems**
  - **Stripping problems**
  - **Absorption problems**
  - **Compaction problems**



# Material Information

- **For each stockpile:**
  - **Quality Level Analysis (QL)**
    - **Are stockpiles crushed or are we guessing?**
    - **How much aggregate is crushed?**
  - **Product Compliance**
  - **Specific Gravities:**
    - $G_{sa}$
    - $G_{sb}$
  - **Same for RAP, lime, and other additives**

# **BATCHING AGGREGATE SAMPLES**

- **Model the HMA C Plant**
- **Dry materials**

# **SELECTING TRIAL ASPHALT CONTENT**

- **Designer's best judgment**

## STAGE 1 RESULTS

- Chances are we didn't hit 4.0% air voids with our WAG on asphalt content.
- “Normalize” the results to predict the volumetrics at 4.0% air voids
  - Math process (paper exercise)
  - Compare the three different blends
- Compare the three normalized results against the specification requirements
- Select one gradation for further testing

# STAGE 1: MIX ADJUSTMENTS

## Adjusting VMA

- **Requires a change in gradation**
- **Change particle shape**
- **Blend Sand**
- **Adjust P200 (dust)**

# STAGE 1: MIX ADJUSTMENTS

## Adjusting Dust-to-Effective

- **Waste Dust (P200)**
- **Blend Change**



## **STAGE 1:**

*The final product of Stage 1 is a single proposed aggregate blend that will be further evaluated in Stages 2 & 3.*

## **STAGE 2: DESIGN ASPHALT CONTENT**

- **Test selected blend with different asphalt contents**
- **Select the asphalt content that gives the desired air voids (normally 4.0%)**
- **Minimum of 4 asphalt contents**

**EXAMPLE:  $P_b$  – 5.0%, 5.5%, 6.0%, and 6.5%**

# CONDITIONING OF SAMPLES

- **Two hours at the compaction temperature**
- **Per AASHTO R 30**



# COMPACTION OF SAMPLES

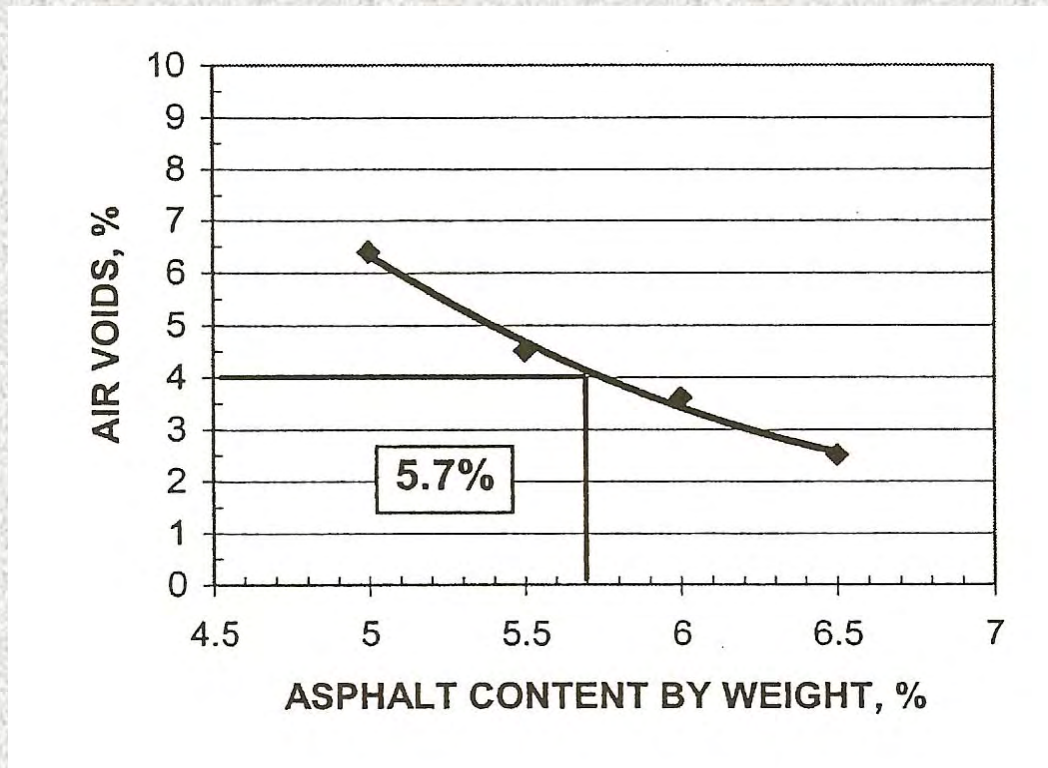


# DESIGN ASPHALT CONTENT

- **Compute the volumetric properties for all four asphalt contents**
- **Data is graphed versus asphalt content:**
  - **Air Voids**
  - **VMA**
  - **VFA**
  - **Dust-to-Effective Asphalt Content**

# DESIGN ASPHALT CONTENT

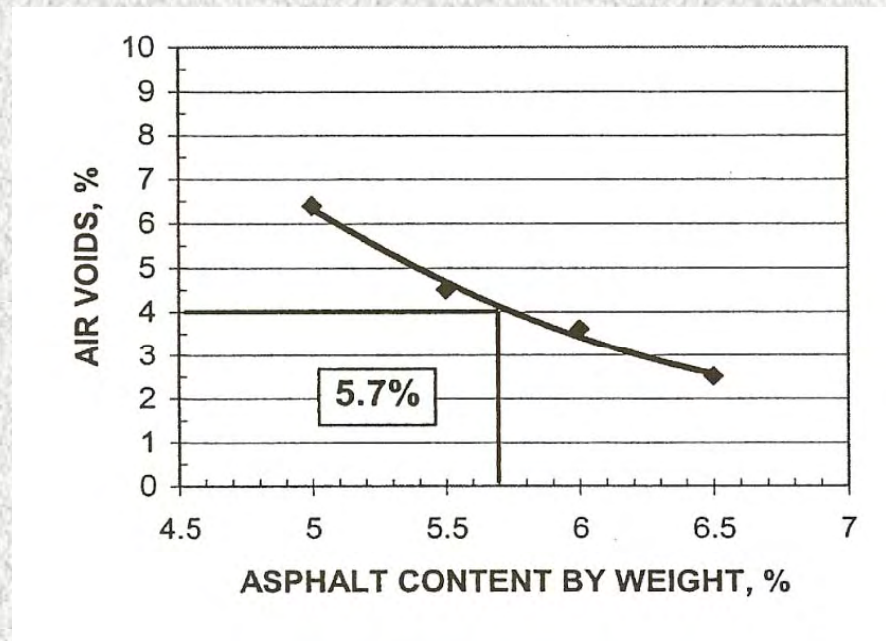
*If the graphs look reasonable, then the design asphalt content is selected as the oil content that provides the target air voids (usually 4.0%) after compaction in gyratory compactor*



# ASSESSING STAGE 2 DATA

## Air Voids vs. Asphalt Cement:

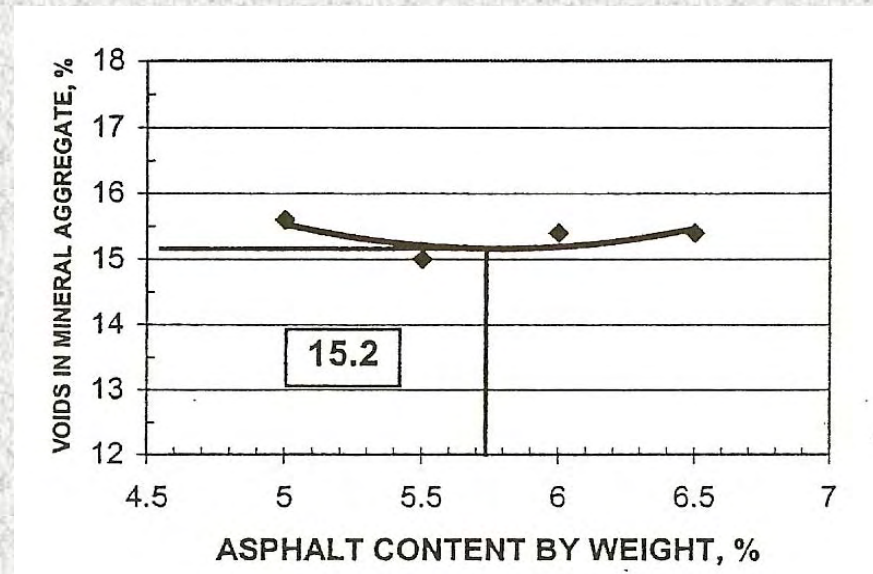
- Smooth curve w/o dips or bumps
- Should match Stage 1



# ASSESSING STAGE 2 DATA

## VMA vs. Asphalt Content:

- Within 1.0 to 1.5% of each other
- Should be a shallow concave curve
- Design near the bottom or on “dry” side

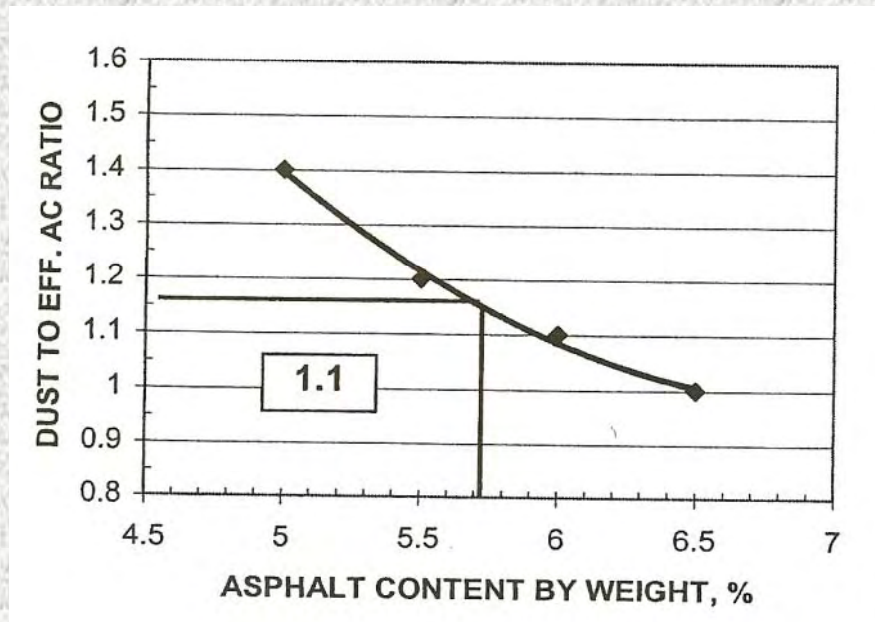




# ASSESSING STAGE 2 DATA

## Dust-to-Effective Ratio vs. Asphalt Content:

- Smooth
- Only thing changing between points is  $P_{be}$
- $P_{be}$  should go up in 0.5% increments



# WHAT MAKES A GOOD DESIGN?

- **Smooth volumetric plots**
- **Stage 1 and Stage 2 match**
- **Design  $P_b$  is on the “dry” side of the VMA curve**
- **Design VMA is 0.5 to 1.0% above the minimum**
- **Design  $P_{be}/P_{200}$  is approximately 1.2**
- **Design TSR above 80**

*The final product of Stage 2 is the design asphalt content.*

# STAGE 3: PERFORMANCE TESTING



- **AASHTO T 283 – Moisture Damage Testing (TSR)**
- **AASHTO T 340 – Rut Susceptibility Testing**
- **Time consuming and expensive**

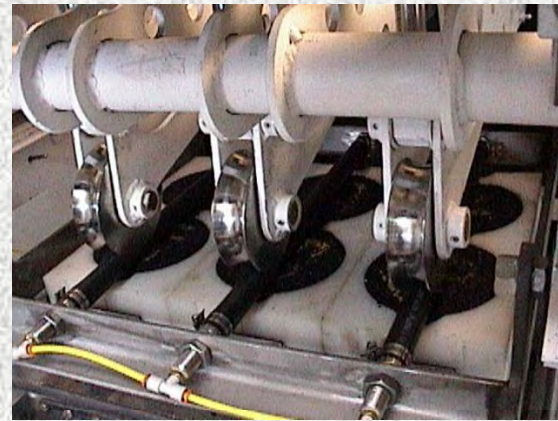
# Rut Susceptibility Testing

**Impacted by aggregate properties:**

- **Particle Shape**
- **Voids in Mineral Aggregate**

**Tested at 64° C:**

- **8000 cycles**
- **100 psi hose**
- **100 lb wheel load**

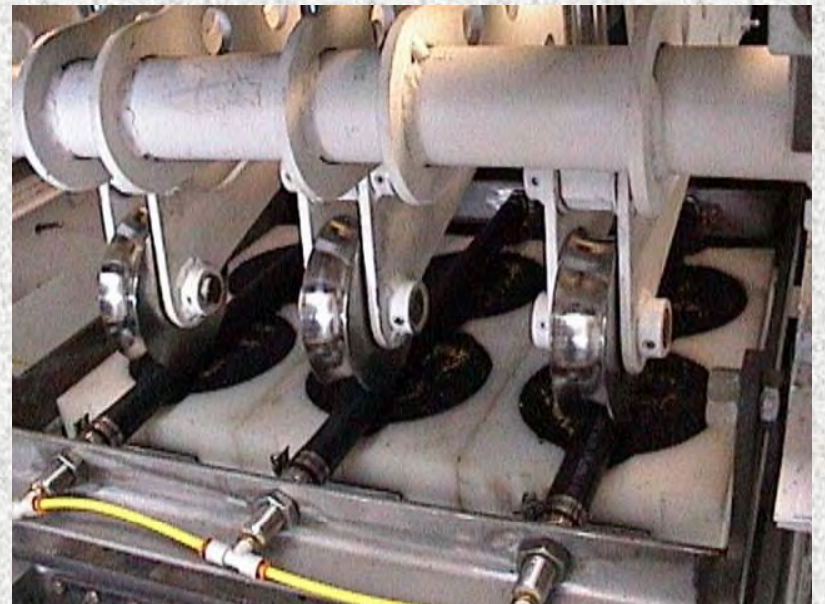


# STAGE 3: PERFORMANCE TESTING

## Rut Susceptibility Testing:

### Mitigating Rut Problems:

- **Add RAP**
- **Increase crushed faces**
- **Eliminate flat and elongated particles**
- **Stiffer grade of asphalt cement**



# Moisture Damage Testing

## Tensile Strength Test (TSR)

- “Stripping Test”
- Stripping occurs in the presence of water and heavy loads
- Mitigated with “anti-strip” or lime



# STAGE 3: PERFORMANCE TESTING

## Moisture Damage Testing (TSR):

- Fabricate 6 gyratory specimens
- Divide in to two subsets: (3 wet and 3 dry)
- Saturate and “torture” the 3 wet specimens in a 140° F hot water bath for 24 hours
- Measure tensile strength of all 6 specimens
- Calculate the ratio of the wet tensile strengths to the dry tensile strengths



# **STAGE 3: PERFORMANCE TESTING**

## **Mitigating a TSR (Stripping) Problem:**

- **Add a liquid anti-strip to the oil (0.25% increments)**
- **Try other brands of oil**
- **Treat the aggregates:**
  - **Lime**
  - **Latex**
- **Contact the Engineer**



# **SUMMARY**

- **Materials selection followed by 3 stage mix design process**
- **Stage 1: Identifies optimal blend of aggregates**
- **Stage 2: Identifies optimal asphalt content**
- **Stage 3: Checks for stripping and rut susceptibility**

# SUMMARY

*Understanding the basics is an important step in being able to recognize a good JMF*

